# **Data Needs Assessment**

Nelson & Washington Counties

US 150

Item No. 4-1068.00 & 4-1069.00



Prepared By: Kentucky Transportation Cabinet (KYTC) Division of Planning & KYTC District 4

October 12, 2010

# **Table of Contents**

I.	I	INTRODUCTION	1
,	۹.	Study Purpose	1
ı	3.	Location	1
II.	ı	PROJECT PURPOSE AND NEED	2
,	۹.	Legislation	2
ı	3.	Project Status	3
(	С.	System Linkage	4
ı	ο.	Modal Interrelationships	5
ı	Ξ.	Social Demands & Economic Development	5
ı	₹.	Transportation Demand	5
(	G.	Capacity	6
ı	Н.	Safety	6
ı		Roadway Deficiencies	7
III.		PRELIMINARY ENVIRONMENTAL OVERVIEW	10
,	۹.	Air Quality	10
ı	3.	Archaeology	10
(	С.	Threatened and Endangered Species	10
ı	ο.	Hazardous Materials	11
ı	Ξ.	Historic Resources	11
ı	₹.	Permitting	13
(	G.	Noise	13
ı	н.	Socioeconomic	13
ı		Section 4(f) Resources	13
J	١.	Section 6(f) Resources	13
IV.		PRELIMINARY PROJECT INFORMATION	13
,	۹.	Existing Conditions/Roadway Data	13
ı	3.	Right of Way	15
(	С.	Utilities	15
	<b>.</b>	Agency Coordination	16

٧.	PROJECT PURPOSE AND NEED STATEMENT	17
VI.	POSSIBLE ALTERNATIVES	17
A.	No Build	17
В.	Build in Place	17
c.	Alternative #1	18
D.	. Alternative #2	19
E.	Alternative #3	20
VII.	Summary	21
	LIST OF FIGURES	
Fig	gure 1: Project Location Map	2
Fig	gure 2: System Linkage Map	4
Fig	gure 3: US 150 Traffic Projection	5
Fig	gure 4: Collision Locations	6
Fig	gure 5: Bridge over Beech Fork Looking East	7
Fig	gure 6: Bridge over Beech Fork (Pier and Beam)	7
Fig	gure 7: Bridge over Cartwright Creek Looking East	8
Fig	gure 8: Under the Bridge over Cartwright Creek	8
Fig	gure 9: Croakes Station Road Intersection	9
Fig	gure 10: Culvert for Beech Fork Overflow	9
Fig	gure 11: Highwater and Drift Accumulation at the Bridge over Beech Fork	10
Fig	gure 12: Date Stamp Found on Both Bridges	12
Fig	gure 13: Site Potentially Eligible for the National Register	12
Fig	gure 14: Preliminary Environmental Footprint	12
Fig	gure 15: Utility Locations	16
Fig	gure 16: Alternative #1	18
Fig	gure 17: Alternative #2	19
Fig	gure 18: Alternative #3	20

# **Table of Contents(Continued)**

### **LIST OF TABLES**

Table 1 USWS Listing of Threatened and Endangered Species in Nelson and Washington Counties	11
Table 2 Existing Conditions and Data Summary	. 14

### LIST OF APPENDICES

Appendix A	Exhibits
Appendix B	UPL Project Information Forms
Appendix C	Collision Data
Appendix D	KYTC's Common Geometric Practices for Rural Arterial Roads
Appendix E	Existing Roadway Plans
Appendix F	Structure Inventory and Appraisal Sheets
Appendix G	FIRM Map(s) of the Study Area
Appendix H	Pictures
Appendix I	Nelson County PVA Map
Appendix J	Project Team Meeting Minutes
Appendix K	Preliminary Cost Estimate Calculations

### I. INTRODUCTION

This study includes two bridge projects, Item Numbers 4-1068.00 and 4-1069.00.

### A. Study Purpose

The purpose of the Data Needs Assessment (DNA) is to address the nine elements of Purpose and Need as defined by NEPA in order to develop a draft Purpose and Need statement for the project(s). This study will also provide a more defined project scope, planning-level cost estimates for possible alternatives, an identification of potential environmental impacts, and other information that will be of assistance in the Project Development phase of this project.

### B. Location

The bridge projects are located closely together near the Nelson-Washington County Line on US 150 (See *Figure 1* and Exhibit 1 in *Appendix A*). Bridge #090B00028N is located over Beech Fork which is also the location of the county line. Bridge #115B00022N is located over Cartwright Creek just east of the Nelson-Washington County Line. There are two county road approaches in the project area, Croakes Station Road and Connor Road. A topographic map of the study area, Exhibit 2, can also be viewed in *Appendix A*.

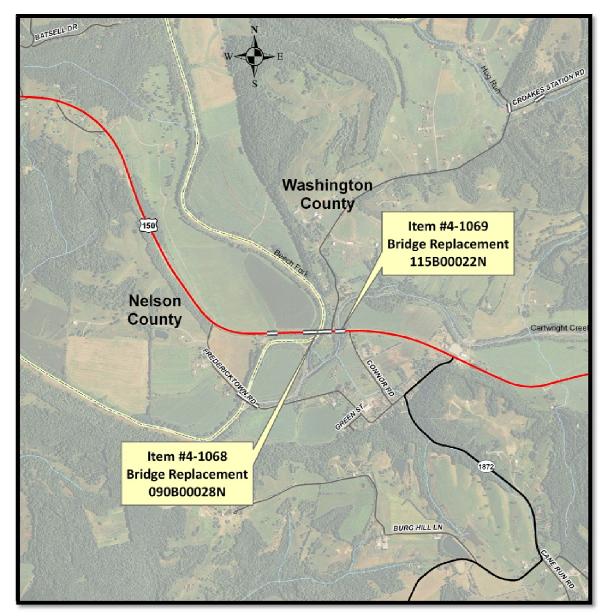


Figure 1: Project Location Map

### II. PROJECT PURPOSE AND NEED

### A. Legislation

The following is a description of the projects as they are listed in the 2010 General Assembly's Enacted Roadway Plan.

### Item #4-1068.00, Nelson County

<u>Phase</u>	<u>Fund</u>	<u>Year</u>	<u>Estimate</u>
D:	BRO	2010	\$490,000
R:	BRO	2012	\$180,000
U:	BRO	2012	\$75,000

REPLACE BRIDGE ON US-150 (MP 7.656) OVER BEECH FORK; ON WASHINGTON - NELSON CL; (STRUCTURALLY DEFICIENT, SR=45.8) 090B00028N

### Item #4-1069.00, Washington County

<u>Phase</u>	<u>Fund</u>	<u>Year</u>	<u>Estimate</u>
D:	BRO	2010	\$250,000
R:	BRO	2012	\$120,000
U:	BRO	2012	\$75,000

REPLACE BRIDGE ON US-150 (MP 0.085) OVER CARTWRIGHT CREEK; .1 MI EAST OF NELSON CL; (STRUCTURALLY DEFICIENT, SR=41.1) 115B00022N

The 2010 Recommended Highway Plan listed Construction cost estimates for Items # 4-1068.00 and 4-1069.00 as \$2,190,000 and \$1,200,000, respectively, for a combined total of \$3,390,000.

### **B.** Project Status

The bridges are structurally deficient with sufficiency ratings of 45.8 and 41.1, as identified above. Design funds have not yet been authorized. The Highway Plan Design year is listed as 2010.

Other Projects in the area include:

- 4-8308.10, Nelson County Widen US-150 from KY-245/Wal-Mart (MP 0.44 to MP 1.697). This project is in the current Highway Plan. Design is scheduled for 2010 with SP funding.
- 4-8309.10, Nelson County Widen US-150 from near KY 245 through the Bluegrass Parkway Interchange to just Past Leslie Ballard Road (MP 1.697 to MP 2.285). This project is in the current Highway Plan. Design is scheduled for 2010 with SP funding.
- 4-307.01, Washington County Construction of the Springfield Northwest Bypass. This project is currently under construction with an expected completion date in 2011.

Projects near the study site on the Unscheduled Projects List include:

- 04 090 B0150 12.00, Nelson County Reconstruction of US 150 from Leslie Ballard Road to the Washington County Line (MP 2.3 to MP 7.682).
- 04 115 B0150 121.00, Washington County Reconstruction from Nelson County Line to Cartwright Creek (MP 0.00 to MP 4.232). This project was ranked High by the district in 2009.

Project Information Forms (PIFs) for these projects can be viewed in **Appendix B**.

### C. System Linkage

US 150 in this area connects Springfield to Bardstown (see *Figure 2* and Exhibit 3 in **Appendix A**). It is a route used by truck traffic coming off of the Bluegrass Parkway. St. Catharine College is also on this route. The completion of US 150 in Rockcastle County may increase traffic from I-75.

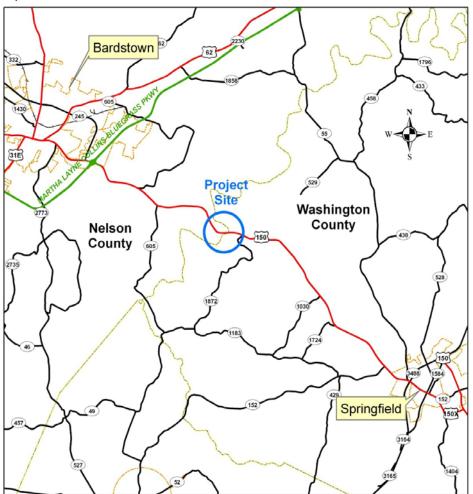


Figure 2: System Linkage Map

US 150 between Bardstown and Springfield has the following roadway classifications:

- Functional Classification Rural Minor Arterial
- State System State Primary
- Scenic Byway Lincoln Heritage Highway
- On the National Truck Network
- Truck Weight Classification AAA
- Not a designated Bike Route

### D. Modal Interrelationships

There is no public transit on this route. The nearest Rail Line is RJ Corman in Bardstown. The amount of traffic generated on this route by the Rail Line is unknown, but is not thought to be substantial. Separate bike/pedestrian facilities are not needed in this area.

### E. Social Demands & Economic Development

Fredericktown Community Park is located just southeast of the project site; however, there is an alternate route into the park. The greatest potential for development that may impact the project site is a 200 acre industrial park on the south side of the Bluegrass Parkway in Bardstown. Currently, only a baker is located in the industrial park.

### F. Transportation Demand

The last actual traffic count at this location was an ADT of 8,290 in 2009. This section of US 150 has generally followed a 3% growth rate with a significant increase sometime between 1992 and 1998. The AADT trend is toward a count of 15,000 in 2030. A more accurate forecast can be requested during Phase I Design. *Figure 3* below displays the trend line based on previous traffic counts.

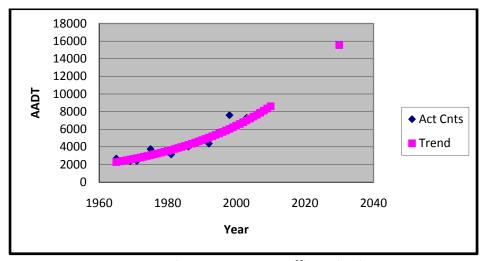


Figure 3: US 150 Traffic Projection

### G. Capacity

The Vehicle/Service Flow (VSF), according to the 2010 Adequacy Rating Data for this section of US 150, is currently 0.46. If the AADT continues to grow at the current rate, consideration may need to be given to increasing the number of through lanes on this corridor to accommodate the 2030 projection.

### H. Safety

Collision data was obtained from the KY State Police database of collisions for a three year period of time from June 1, 2007 to May 31, 2010. There were 12 collisions reported in the project area during this three year time period. Four of the collisions were located at the intersection with Connor Road. Two were located at the intersection with Croakes Station Road. All but one of these occurred at night and, in the description of the collisions in the reports, two of them stated that sight distance was limited by the bridge railings. The manner and location of the collisions can be viewed in *Figure 4*. Weather did not appear to be a significant factor in the collisions. A 0.3 mile spot analysis was done at the project site which resulted in a 0.79 Critical Rate Factor. A more detailed analysis of the collision data can be seen in **Appendix C**.

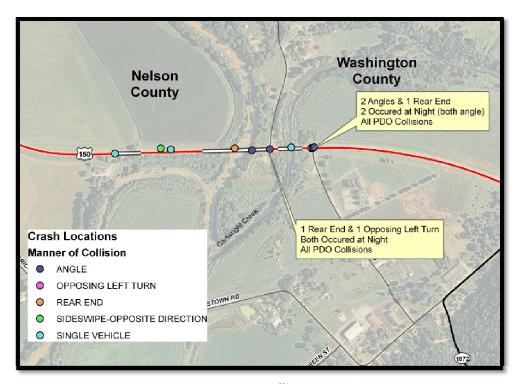


Figure 4: Collision Locations

### I. Roadway Deficiencies

Within the project limits, the roadway currently has 11-ft lanes, 4-8 ft shoulders with guardrail on both sides of the road, approximately a 0% grade, a posted speed limit of 55 MPH, and an Adequacy Rating Percentile of 85.7. KYTC's Common Geometric Practices for Rural Arterial Roads (see **Appendix D**) for this type of road recommends 12-ft lanes for a 60 MPH Design Speed and 8-ft shoulders. Existing roadway plans for this roadway can be viewed in **Appendix E**.

The bridge over Beech Fork is 404.9 feet long and 33.1 feet wide out to out (27.9 feet wide curb to curb). It is structurally deficient with a sufficiency rating of 45.5 and does not meet the guidelines stated above of 12-ft lanes and 8-ft shoulders. A Structure Inventory and Appraisal Sheet for this bridge can be found in **Appendix F**. Photographs of this bridge can be seen below in **Figures 5** and **6**.



Figure 5: Bridge over Beech Fork Looking East



Figure 6: Bridge over Beech Fork (Pier and Beam)

The bridge over Cartwright Creek is 225.1 feet long and 30.5 feet wide out to out (27.6 feet wide curb to curb). It is structurally deficient with a sufficiency rating of 40.8 and does not meet the guidelines stated above of 12-ft lanes and 8-ft shoulders. A Structure Inventory and Appraisal Sheet for this bridge can be found in **Appendix F**.



Figure 7: Bridge over Cartwright Creek Looking East



Figure 8: Under the Bridge over Cartwright Creek

Although these bridges are located in a flat, tangent section of roadway, there may be sight distance problems at the intersections of each of the county roads in the project limits. As was stated in the previous section of this report, there were four collisions reported at the intersection with Connor Road and two collisions reported at the intersection with Croakes Station Road. According to the accident reports the bridge railing may have limited the sight distance for drivers turning onto US 150 from the

county roads. A picture of the Croakes Station Road intersection can be seen in *Figure 9*.



Figure 9: Croakes Station Road Intersection

It should also be noted that there is a 46-ft long, three-span culvert located approximately 500 feet west of the bridge over Beech Fork. The culvert is dry most of the time, and is used to accommodate the overflow from Beech Fork. It is not structurally deficient, but does have some issues with the wing walls separating from the culvert and some rebar exposure. A picture of the culvert can be seen in *Figure 10*.



Figure 10: Culvert for Beech Fork Overflow

Flooding over the bridges has not been reported, but, as can be seen in *Figure 11*, water has risen to the superstructure and there is a problem with conveyance. There is a problem with debris catching on the piers in this location. A floodway analysis will need to be performed in future project phases to determine the needed hydraulic opening for the water under the bridges. Flood Insurance Rate Maps (FIRM) of the project area are located in **Appendix G**. Additional pictures of the project site are in **Appendix H**.



Figure 11: Highwater and Drift Accumulation at the Bridge over Beech Fork

### III. PRELIMINARY ENVIRONMENTAL OVERVIEW

### A. Air Quality

Nelson and Washington County are in attainment for all monitored air pollutants.

### B. Archaeology

An archaeology Phase I survey will need to be completed in order to rule out any impacts to archaeological sites.

### C. Threatened and Endangered Species

The USFWS has identified the known and potential presence of threatened and endangered species in Nelson and Washington Counties (see Table 1). During a site visit

in July 2010 potential habitat was observed for the bat species in the riparian corridor. Additionally, several middens of a variety of different mussel species were observed along the bank below the Beech Fork Bridge. A biological assessment should be completed prior to construction to assess the potential impact to threatened and endangered species.

Table 1 – USFWS listing of Threatened and Endangered Species in Nelson and Washington Counties

J	1 Counties.		Logol*				
Group	Species	Common name	Legal* Status				
Nelson Cou	Nelson County						
Mammals	Myotis grisescens	gray bat	Е				
	Myotis sodalis	Indiana bat	Е				
Mussels	Pleurobema clava	clubshell	Е				
	Cyprogenia stegaria	fanshell	Е				
	Epioblasma torulosa rangiana	Northern riffleshell	E				
	Lampsilis abrupta	pink mucket	Е				
	Plethobasus cooperianus	orangefoot pimpleback	Е				
Plants	Apios priceana	Price's potato-bean	Т				
	Trifolium stoloniferum	running buffalo clover	Е				
Washington County							
Mammals	Myotis sodalis	Indiana bat	Е				
Mussels	Pleurobema clava	clubshell	Е				
	Cyprogenia stegaria	fanshell	Е				

<sup>\*</sup>E- Endangered, T- Threatened

### D. Hazardous Materials

During a site visit on July 16, 2010, no properties were observed that would have a high probability of hazardous materials. However, due to the age of the bridges the material used to seal the joints should be tested for asbestos prior to demolition.

### E. Historic Resources

The two bridges were constructed during the 1950s; this allows them to meet at least the first screening requirement for listing on the National Register of Historic Places (see *Figure 12*). Additionally, during a site visit on July 16, 2010 a conversation with a local property owner revealed that the closest residence to the existing bridges was built in the 1920s making it potentially eligible for listing on the National Register of Historic Places (see *Figure 13*). It is unlikely that the house itself will be impacted, but there is a potential to impact the property on which it is located. Therefore, a thorough assessment of the eligibility of the bridges and the local residence should be completed in future project phases. *Figure 14* indicates the location of the residence and other areas of potential environmental concern.



Figure 12: Date Stamp Found on Both Bridges



Figure 13: Site Potentially Eligible for the National Register

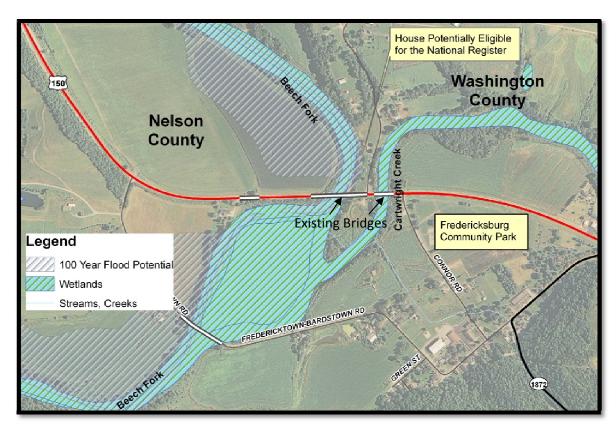


Figure 14: Preliminary Environmental Footprint

### F. Permitting

Any impacts below the ordinary high water mark within either Beech Fork or Cartwright Creek will need a USACE 404 permit.

### G. Noise

The scope of the project should not require additional noise analyses since there are no additional lanes of traffic planned for the facility.

### H. Socioeconomic

Socioeconomic impacts could occur if significant impacts occur to the Fredericksburg Community Park.

### I. Section 4(f) Resources

The Fredericksburg Community Park is protected under Section 4(f) of the Department of Transportation Act of 1966. Additionally, if either the bridges or residences located nearby are ruled as eligible for the National Register of Historic Places they could also be afforded protection under Section 4(f). The Kentucky Transportation Cabinet (KYTC) has options to mitigate and avoid impacts to section 4(f) resources including a programmatic agreement for mitigating historic bridges, using 'de minimus' guidance for minor strip takings.

### J. Section 6(f) Resources

The Fredericksburg Community Park was partially funded by the Land Water Conservation Fund; therefore, is afforded protection under Section 6(f) of the Land Water Conservation Fund Act. This Act states that grant-assisted areas are to forever remain available for "public outdoor recreation use," or be replaced by lands of equal market value and recreation usefulness. If the Fredericksburg Community Park is affected by Right of Way acquisition the KYTC will be required to mitigate these impacts through additional land purchase for the park.

### IV. PRELIMINARY PROJECT INFORMATION

### A. Existing Conditions/Roadway Data

A summary of the existing conditions can be seen in Table 2.

County(ies): Nelson, Washington

Springfield/Bardstown

Route Number(s): <u>US 150</u> Road Name: <u>Rd.</u>

Item No.: <u>04-1068, 04-1069</u>

BMP:  $\sim$  7.4 Nelson Co. EMP:  $\sim$  0.2 Washington Co.

Project Length: < 1 mile

Rdwy. Class.: <u>Rural Minor Arterial</u> State Class.: <u>Primary</u>

Truck Class: AAA
ADT (current): 8430

Terrain: Rolling Access Control: Permit
Posted Speed: 55 MPH Median Type: Undivided

Funding Type: BRO

### **Roadway Data:**

	Existing Conditions	<u>Design Citeria</u> *
No. of Lanes	2	2
Lane Width	11 ft	12 ft
Shoulder Width	4-8 ft	8 ft
Minimum Radius	-	1205 ft
Maximum Grade	0%	4%
Adequacy Rating		* 60 MPH Design Speed

racquacy manns

%: 85.7

### **Bridge Data:**

	<u>090B00028N</u>	<u>115B00022N</u>
Max. Span Length	89.9 ft	89.9 ft
Length	404.9 ft	225.1 ft
Width, out to out Width, curb to	33.1 ft	30.5 ft
curb	27.9 ft	27.6 ft
Sufficiency Rating	45.5	40.8

US 150 DNA Nelson, Washington Counties

It should be noted that just west of the project site, the alignment follows a steep grade, approximately 4.3%, down to the project site. The project site has guardrail on both sides of the road due to steep side slopes. The section of the roadway in the project area is straight with a 0% grade.

### B. Right of Way

According to the Property Value Administrator (PVA) information available online for Nelson County and the right of way information available on the set of plans for the existing roadway, there are potentially seven properties that could be impacted by this project. The PVA information available online for Nelson County can be seen in **Appendix I**.

### C. Utilities

Electric: Salt River Electric

Mr. Gary Pile, Engineer 111 West Brashear Ave. Bardstown, KY 40004

502-348-3931

Telephone: AT & T KY

Ms. Brenda Richards, Specialist

1535 Twilight Trail Frankfort, KY 40601 502-875-5983

Water: City of Bardstown

Steve Hicks, Asst. Dir. Of Public Works

220 North 5<sup>th</sup> Street Bardstown, KY 40004

502-249-1176

The project team confirmed that there are no gas or sewer lines near the project site. A preliminary sketch of the approximate location of the utilities in the project area can be viewed in *Figure 15*. This information was obtained from field inspection and an ARC GIS database. Confirmation of these locations should be verified as the project survey is completed in the Design phase.

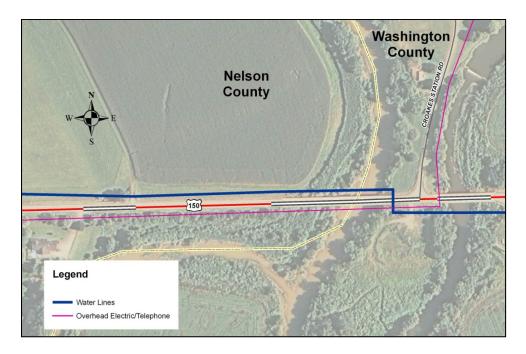


Figure 15: Utility Locations

### D. Agency Coordination

The Project Team met on June 16, 2010 to review and discuss the projects and the Pre-Design Scoping Study. The team discussed alternatives. Due to the 6(f) property, Fredericktown Community Park, and the location of the tributary to Beech Fork located on the south side of the existing alignment, the project team recommended moving the alignment to the north. The project team also agreed that turning lanes were not needed for either of the intersection of the county roads with US 150. A width of 40-ft curb to curb was recommended for the bridges for estimate purposes. A typical of 12-ft lanes and 8-ft shoulders was recommended for approaches to meet the 60 mph Design Speed guidelines. Section VI if this report discusses possible alternatives that were a result of information gathered from the project team meeting, the site visit, and other information obtained for this project.

The deficiencies of the bridges were discussed. The opening will need to be studied hydraulically during Phase I Design. It was suggested that the alignment be raised to increase the size of the hydraulic opening. Moving the pier(s) to allow for a longer span (currently 90 feet) may also be helpful, and will need to be considered during the hydraulic analysis. More details of deficiencies were discussed in Section II.I. of this report.

The minutes of the meeting can be reviewed in **Appendix J**.

### V. PROJECT PURPOSE AND NEED STATEMENT

Based upon the information presented in Section II of this report and discussion of the project team, the following purpose and need statement was drafted for these projects:

US 150 provides a vital connection between the city of Bardstown and Springfield. The bridges located over Beech Fork on the Nelson-Washington County Line and the bridge over Cartwright Creek just east of the County Line are structurally deficient. There are collisions occurring at the intersections of Croakes Station Road and Connor Road that appear to be occurring due to poor sight distance at the intersections. There are also conveyance problems with the existing structures and the bridge piers accumulate large amounts of debris. The purpose of this project is to address the structural deficiencies and conveyance issues of the bridges, and the occurrence of collisions at the intersections in order to provide safety, mobility and connectivity between Springfield and Bardstown.

### VI. POSSIBLE ALTERNATIVES

The following is a description of several of the alternatives analyzed and discussed during the development of this study. Preliminary cost estimate calculations can be viewed in **Appendix K**.

### A. No Build

The No Build option is not a feasible alternative due to the structural deficiency of the bridges. It would not address the draft purpose and need defined for of these projects.

### B. Build in Place

There are a couple of options with the Build in Place alternatives; however, they are not feasible. The terrain is not favorable for two low-water crossings and a detour using state routes and closing US 150 would require motorists to travel more than eight additional miles.

### C. Alternative #1

This alternative involves moving the new structures several feet north of the existing alignment with a new, parallel alignment. This may require a replacement of the culvert west of the bridges to accommodate the tie-in of the approaches to the new bridge. The culvert is not currently structurally deficient, but does have some issues with separation of the wing walls from the culvert headwall and some exposure of rebar. In addition, it is suggested that the alignment be raised and/or the span length be increased to increase the hydraulic opening of the bridges. It was also recommended that current design standards be used (12-ft lanes, 8-ft shoulders) on both the approaches and the bridges, which would require the bridges to be 40-ft curb to curb. This option would allow for two lanes of traffic to remain open while constructing the bridges. The length of the project will vary depending on decisions made in Phase I Design, but should be less than a mile and will include roadway widening and at least two new structures. The size of the first bridge will be approximately 40 feet curb-tocurb by 405 feet long and the second bridge will be approximately 40 feet curb-to-curb by 90 feet long. This alternative may also require construction of a new culvert depending on how far to the north the alignment is moved. The size and location of the culvert will depend on the location of the new alignment. This alternative will require the purchase of right of way, utility relocation, a significant amount of fill, and the reconstruction of two field entrances and two entrances to county roads. A sketch of this alternative can be viewed below in Figure 16.

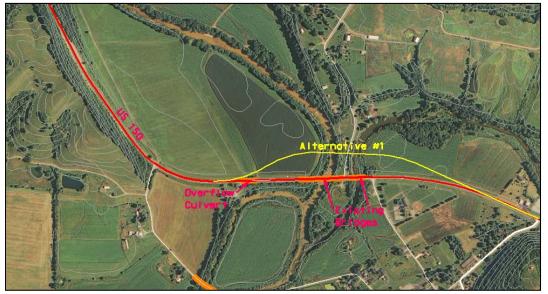


Figure 16: Alternative #1

The following is the preliminary cost estimated for Alternative #1:

 Phase
 Estimate

 Right of Way
 \$300,000

 Utilities
 \$75,000

 Construction
 \$6,000,000

### D. Alternative #2

Another option is partial width construction of the new bridges which would shift the center line approximately 7 feet to the north in order to accommodate the proposed lane widths and shoulder widths of 12 feet and 8 feet, respectively (see Figure 17). For this alternative, the outside edge of the right (south) shoulder on the bridges would be held and all widening would occur to the north of the existing structure. This would allow shorter tie-ins to the approaches and entrances, and would require a culvert extension of approximately 11 feet to accommodate the shift in the alignment and the widening of the roadway and shoulders. Raising the elevation of the alignment would still be possible. This option would have a minor impact on right of way, and would require the road width to be reduced to one lane during construction with a temporary traffic signal to control the direction of traffic flow. The width needed for traffic is 17 feet (12-ft lane width + 2 feet for the barrier + 3 feet for the overhang). The length of the project may be approximately 3000 feet including roadway widening, a culvert extension (around 11-ft wide by 46-ft long, triple barrel), and two new structures (approximately 40 feet curb-to-curb by 405 feet long and 40 feet curb-to-curb by 90 feet long).

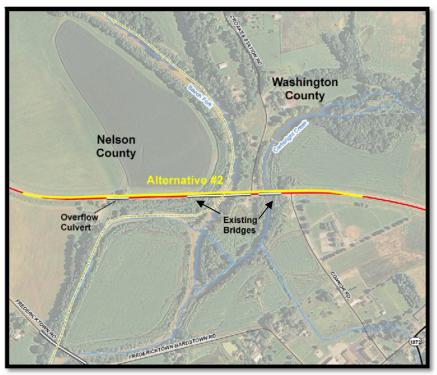


Figure 17: Alternative #2

The following is the preliminary cost estimated for Alternative #2:

<u>Phase</u>	<u>Estimate</u>	
Right of Way	\$100,000	
Utilities	\$150,000	
Construction	\$5,000,000	

### E. Alternative #3

A similar option to Alternative #2, if constructible, is partial width construction of the new bridges which would keep the same center line, but would shift it with a taper temporarily for construction. For this alternative widening the roadway to accommodate 12-ft lanes and 8-ft shoulders would only occur on the two new structures and the segment of roadway between them; the widening would occur on both sides of the centerline (approximately 5 feet on each side for roadway, and 7 feet on each side for the bridge). The roadway and shoulders would taper down at the end of each approach to match existing widths. Temporarily there would need to be slight detour (50:1 taper) to the north while construction on the south side of the bridges occurs. Fill material would be required for the detour as well as the roadway widening. This would not require the extension of the culvert to the west of the bridges. This option would most likely have the least impact on right of way, but would require the road width to be reduced to one-lane during construction with a temporary traffic signal to control the direction of traffic flow. The width needed for traffic is 17 feet (12-ft lane width + 2 feet for the barrier + 3 feet for the overhang). The length of the project may be approximately 1500 feet including roadway widening between bridges and lane width tapers at each end. This alternative includes two new structures approximately 40 feet curb-to-curb by 405 feet long and 40 feet curb-to-curb by 90 feet long.

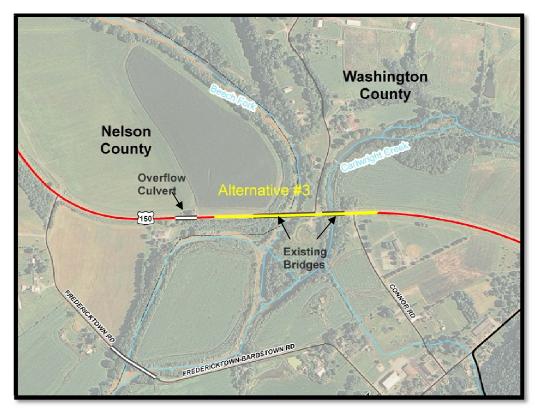


Figure 18: Alternative #3

The following is the preliminary cost estimated for Alternative #3:

<u>Phase</u>	<u>Estimate</u>
Right of Way	\$100,000
Utilities	\$150,000
Construction	\$5,000,000

### VII. Summary

This study is a Data Needs Assessment (DNA) of two projects located on US 150 at or near the Nelson-Washington County Line. Bridge #090B00028N is located over Beech Fork which is also the location of the county line. Bridge #115B00022N is located over Cartwright Creek just east of the Nelson-Washington County Line. Through analysis of existing roadway geometrics, bridge ratings, crash data, site visits, and discussion with the project team the following needs were identified:

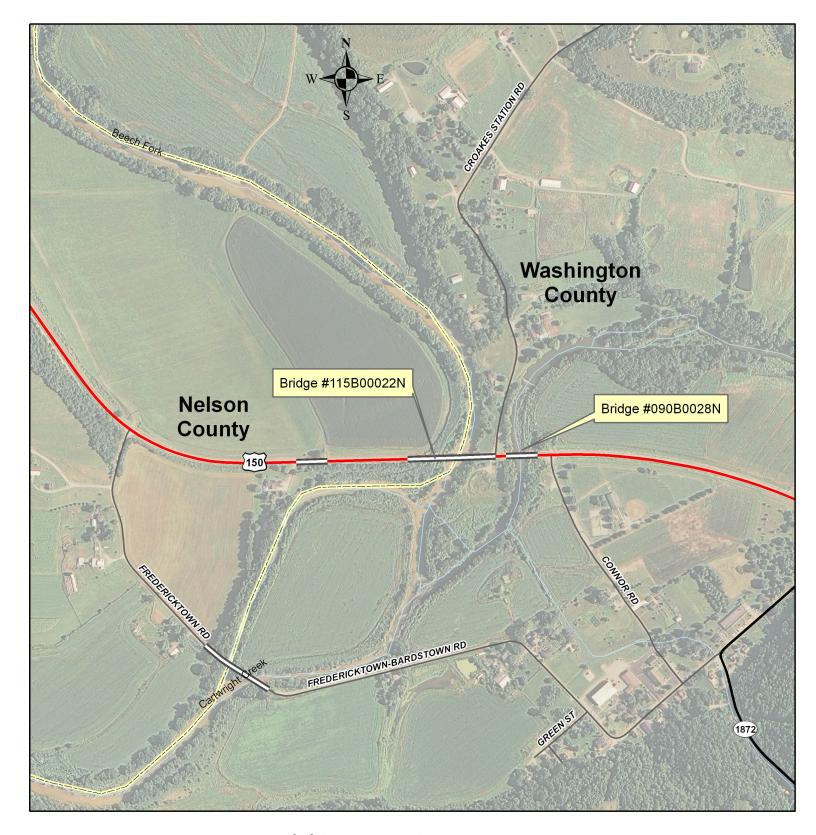
- The bridge located over Beech Fork on the Nelson-Washington County Line and the bridge over Cartwright Creek just east of the County Line are structurally deficient.
- There are collisions occurring at the intersections of Croakes Station Road and Connor Road that appear to be due to poor sight distance at the intersections near the bridges.
- There are also conveyance problems with the existing structures and the bridge piers accumulate large amounts of debris.

The purpose of this project is to address the structural deficiencies and conveyance issues of the bridges and the occurrence of collisions at the intersections in order to provide safety, mobility and connectivity between Springfield and Bardstown.

Three possible alternatives for replacing the bridge are included in this study. One alternate moves the bridge over to a slightly different alignment. Two of the alternates involve the use of partial width construction. All of the alternates include a wider typical with shoulders which would allow for more sight distance at the intersections with the county roads. Increasing or modifying the spacing of the bridge piers and raising the elevation of the beams to allow for a larger hydraulic opening was also discussed. The preliminary construction cost estimates for these alternates ranged from \$5 million to \$6 million which includes the replacement of both bridges. This should be taken into consideration when programming the construction phase of these projects in the next Highway Plan.

For more Information Contact:
Kentucky Transportation Cabinet
Division of Planning, 5<sup>th</sup> Floor West
200 Mero St.
Frankfort, KY 40622

# Appendix A - Exhibits



# Legend

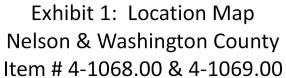
==== Bridge

US Highways

State Roads

—— Local Roads

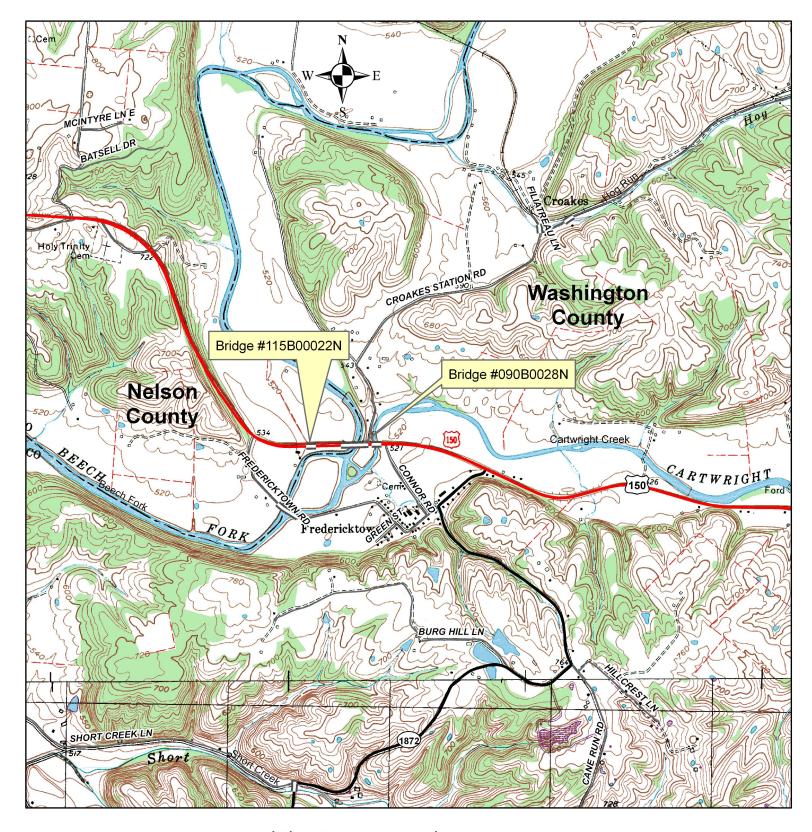
---- County Boundary Lines











# Legend

Bridge

US Highways

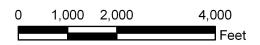
---- State Roads

—— Local Roads

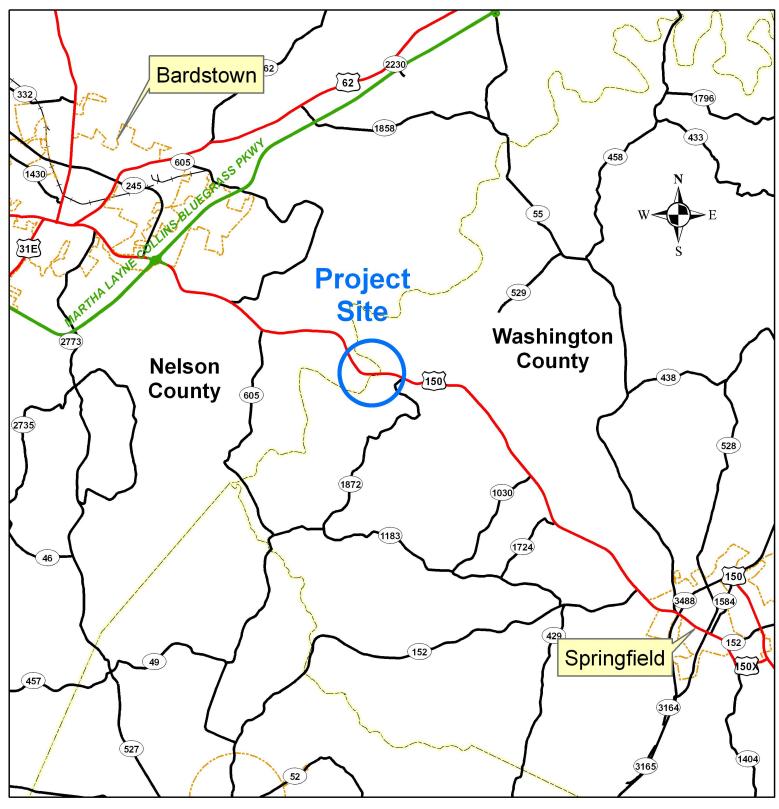
---- County Boundary Lines

Exhibit 2: Topographic Map Nelson & Washington County Item # 4-1068.00 & 4-1069.00









# Legend

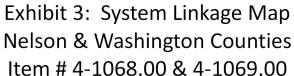
—— Parkways

US Highways

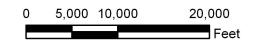
State Roads

County Boundary Lines

---- Corporate Boundary Lines









# Appendix B – UPL Project Information Forms

## KYTC Project Identification Form

Cycle Year: <u>07</u>

	KYICPI	roject laentification l				
PIF Revised: Aug. 2004			Prior Tier:		<u>led</u> D: <u>Hi</u>	
					D:	
			Over	rall Top Ten: R:	D:	
Section I – General Info	ormation	UPL Control #: 04 (	090 B0150 12.00	Co. #: <u>0</u>	100	
Requested by:	Judge Settles		15 B0150 118.00	<u>΄</u> – Co. π. <u>u</u>	190	
Title/Organization:	Washington Co.	RSE Unique Number: 090	US-150			
Date:	12/20/06					
Date.	12/20/00	District: 4	County: Nelson	Route	: US-150	
		ADD: <u>LTADD</u>	MPO: <u>n/a</u>	SUA:		
Form Completed by:	Malham/		g g	G B		
Title/Organization:	LTADD/KYTC-D4	Mode: <u>Highway</u> Type: <u>Reconstruction</u>	State Sy Funct'l			
Date:	12/21/06	Type. <u>Reconstruction</u>	Tunet 1	Class. Kurariv	IIII AI t	
		Project Length: 5.382		ost Estimate: \$ <u>36,8</u>		
Revision 1 by:	Malham/		(P:300 D:3,000	R:2,500 U:1,000	C:30000)	
Title/Organization:	LTADD	Possible Funding Sources (	Check all that apply)	):		
Date:	11/28/07	IM NH HES			□CMAQ	
Revision 2 by:		□PLH □Other:				
Title/Organization:		Highway Networks (Check	all that apply):	⊠Non NHS	□NHS	
Date:		NN Scenic Byw				
		☐ Defense ☐ Strahnet	□Ext. Wt.	☐ADHS ( )		
		E-:-4: D:4 C44: (V	)-			
Section II – Problem St	atement	Existing Project Studies (Yo			D 0	
Route Number: <u>US-150</u>		(Use Report Year)	Original	Rev. 1	Rev. 2	
Beginning MP: <u>2.300</u> Ending MP: <u>7.682</u>		AdequacyRating:  • CRF: (Year)	87.50: (05) 0.89: (05)	87.50: (06) 0.56: (06)	:( )	
Total Length: 5.382		• IRI: (Year)	107: (05)	106: (06)	:( )	
<u> </u>		V/SF: (Year)	0.46: (05)	0.47: (06)	:( )	
Primary Purpose: Upgrade Existing System(Major)		Current ADT: (Year):	10,900: (06)	11300: (07)	:( )	
		Percent Trucks: (Year):	:()	:( )	:( )	
		Projected ADT (HDO): Year	r: %Grow	rth: AD	1:	
Please provide a clear pro	oblem statement for this p	project:				
US-150 is the primary r	oadway from the Blueg	rass Parkway to the City	of Springfield lo	ocated in Washi	ngton County.	
	·	oncerns. Current ADT ra			•	
		andard 3 indicating infre				
speed issues. The Six-Y	ear Plan contains a pro	ject (4-8309) to widen US	-150 from near	KY 245 through	the	
		e Ballard Road (MP 2.3).		0		
·						
Section III – Project Des	scription					
Project Description Narr	ative:					
			_			
Reconstruction of US 1	50 from Leslie Ballard l	Rd to the Washington Co	unty Line.			

Regional Goals/Objectives Addressed: III-Preserve, maintain, and enhance the existing transportation system to ensure safe, efficient, and effective mobility.

UPL#:	04 090 B0150 12.00				
	County: 1	Nelson Co	#.	090	Route: US-150

**Section IV – Project Area Information:** 

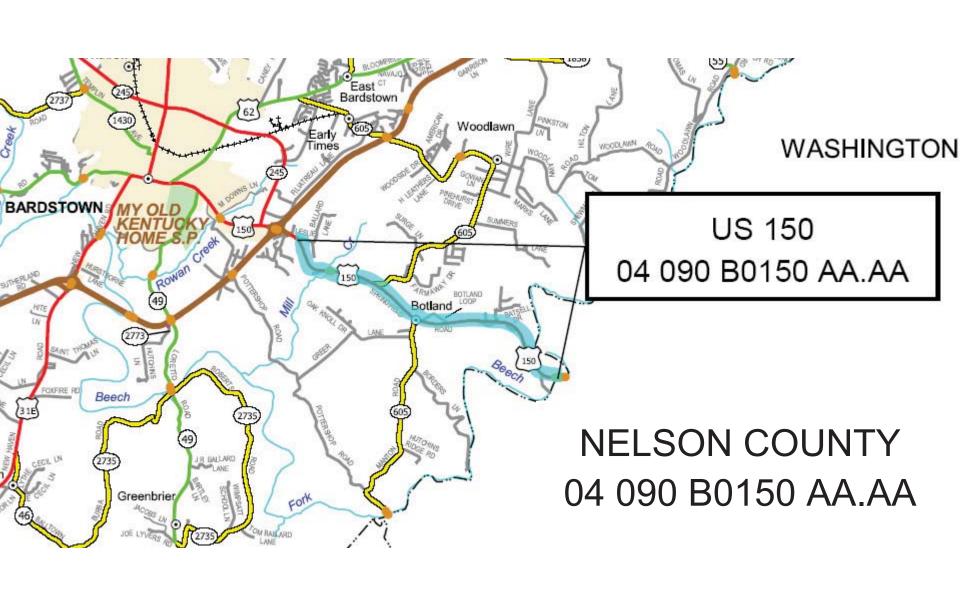
1.	Miscellaneous Roadway	Access Control:	Existing: Permit		Median Type:	Existing:	Width: <u>0'</u>	
	Conditions		Proposed:			Proposed:	Width:	
		Lane	Existing: <u>2/11'</u>		Shoulders:	Existing: <u>Asphalt</u>	Width: <u>4-8'</u>	
		No./Width:	Proposed: <u>2/12'</u>		Shoulders.	Proposed: Asphalt	Width: <b>2-10'</b>	
		No. of Bridges:	Existing: <u>1</u> Proposed: <u>1</u>		Other Improvement Projects in Area:	□None ⊠SYP □Resurfa	ice	
		Comments:	Troposed. <u>I</u>		Trojects in Area.			
2.	Right of Way	Avg. Width: Exist	ting: <u>100'</u>	Source: X HI	S Plans M	icrofilm Other		
		Current Primary Use:						
		☐ No ☐ Yes Project may require additional R/W. Possible Relocations : Homes: Businesses:						
		Comments:						
3.	Utilities	Existing Utilities:						
			D :	T. 11. 15. 1	Comments:			
		☐ No ⊠ Yes	Project may require U	Itility Relocati	ions.			
4	Environmental	mental (Check all that apply):						
	Impacts	□ Blueline Streams       □ Wetlands       □ Floodplain       □ Wildlife Managed Areas       □ Historic Properties         □ Cemeteries       □ Schools       □ Churches       □ Endangered Species       □ Public Land/Park         □ Noise Impact       □ Arch. Sites       □ NR Properties       □ Potential NR Properties       □ Other:						
		Potential Conta	aminated sites:	Gas Stations	Landfills		ards Other	
		Comments:					<del>_</del>	
5.	Air Quality	No ☐Yes Project is located in a Maintenance or Nonattainment Area ☐ Ozone ☐ PM 2.5						
		No ☐Yes Project adds through lane capacity						
		No ☐Yes Project results from a Congestion Management Plan						
		No ☐Yes Project is included in TIP/STIP TIP Page # STIP Page #						
		Comments:						
6.	Economic	□No ⊠Yes	Planning/Zoning Reg	gulations	□No ⊠Y			
	Impacts	☐ No ⊠ Yes	exist in Community Commercial or Industrial Districts.  This project has economic impacts on regional/local economy:					
						ity Retail Sales Other		
		□ No □ V	Please Describe:	dinant	to maion maiorta a C	tomosti		
		□ No ⊠ Yes	This project provides direct access to major points of interest:  Nat'l/State Parks Monuments Historic Sites Amusement Parks US Public Land Other					
			Please Describe: My O					
		□ No ⊠ Yes	This project provides  ⊠ Shopping Centers  ∑					
			DI D 1					

Page 2 of 3 Filename: 04 090 B0150 12^00

UPL #:	04 090 B0150 12.00			
	County: Nelson	Co. #:	090	Route: US-150

						County: Iveisor	1 Co. #: 090 Ro	-utc. 05 150
7 75 111	T						_	
7. Multimodal Opportunities	This project is a candid	date for: (ch	eck all that apply)		cycle Patl rk/Ride L	ns Sidewalks ots N/A	Shared-Us	se Paths
	This project improves	direct acces	ss to: (check all that a			Railways utes N/A	Riverports	3
	Type of Public Transp	ortation ava	ailable:	☐ Fix	ked Route	e Demand Respo	nse	
	Comments:							
8. Social Impacts	This project may affec (Check all that apply)		Neighborhood or C Travel Patterns (Vo Household Relocat Elderly, disabled, r No adverse effects	chicular, com ions ondrivers, m	muter, bi inorities,	low-income persons		
	Comments/Impact Des							
Cost Estimate by I					, n	D :: a	D (	l p
Phase Planning	Original Estimate \$300,000	By:	Revision 1	Date	By:	Revision 2	Date	By:
Design	\$3,000,000	JH						+
ROW	\$2,500,000	JH						+
Utilities	\$1,000,000	JH						+
Construction	\$30,000,000	JH						+
Total Cost	\$36,800,000	JH						+
Total Cost	ψ30,000,000	011						
<b>Estimate Procedur</b>	e Used:							
Origina	l Estimate:		Revision	1:		Revi	ision 2:	
Per Mile@	\$		Per Mile@\$	_		Per Mile@	\$	
Terrain:			Terrain:	_		Terrain:		
Detailed Est			Detailed Estimate Calculations Atta			Detailed Est Calculations		
Estimate Assumption See Rev. 1	ons:	-Project should climbin	e Assumptions: et should widen le ers, provide turn ng lanes where ne e alignment.	lanes and	truck	Estimate Assumptio	<u>ns</u> :	
Estimate Class: E-Rec Section VI – Attac	<u> </u>	Estimate	c Class:			Estimate Class:		
	s are attached to this do	cument:	□ Location Map     □	Photog	graph(s)	Other:		
Comments:								

Page 3 of 3 Filename: 04 090 B0150 12^00





# KYTC Project Identification Form

Cycle Year: <u>07</u> Priority: L: \_\_\_\_ R: Hi

Tier:

D: <u>**Hi**</u>

	. ,•			Rank: R: _ rall Top Ten: R: 5	D:
Section I – General Info	ormation	UPL Control #: 04	115 B0150 121.0	<b>0</b> Co. #: <u>115</u>	
Requested by:	Judge Settles	Parent Control #: 04 1	115 B0150 118.00		
Title/Organization:	Washington Co.	RSE Unique Number: 115	US-150		
Date:	12/20/06				
Dutc.	12/20/00	District: 4	County: Washing	gton Route	e: <u>US-150</u>
		ADD: <u>LTADD</u>	MPO: <u>n/a</u>	SUA:	<u>n/a</u>
Form Completed by:	Malham/	Mode: <b>Highway</b>	Ctata Cv	estamo Stata D	
Title/Organization:	LTADD/KYTC-D4	Type: Reconstruction	State Sy Funct'l		
Date:	12/21/06				
		Project Length: 4.232		ost Estimate: \$ <u>28,</u>	
Revision 1 by:	Malham/K Young		(P:250 D:2,500	R:2,000 U:1,000	C:23000)
Title/Organization:	LTADD/KYTC-D4	Possible Funding Sources (		:	
Date:	11/24/08		S □BR ⊠STP	⊠SP □TE	□CMAQ
Revision 2 by:		PLH Other:			
Title/Organization:		Highway Networks (Check	all that apply):	⊠Non NHS	□NHS
Date:		NN ☐ Scenic Byw	· =	Bike	Forest
		☐ Defense ☐ Strahnet	□Ext. Wt.	□ADHS ( )	
Section II – Problem St	tatamant	Existing Project Studies (Y	ear):		
Route Number: US-150	iatement	(Use Report Year)	Original	Rev. 1	Rev. 2
Beginning MP: $0.000$		AdequacyRating:	85.50: (05)	91.00: (06)	:( )
Ending MP: 4.232		CRF: (Year)	0.48: (05)	0.42: (06)	:( )
Total Length: $\underline{4.232}$		<ul><li>IRI: (Year)</li><li>V/SF: (Year)</li></ul>	107: (05) 0.39: (05)	114: (06) 0.41: (06)	:( )
Daimana Damana and Umana da	Evistina Crustom (Maion)	Current ADT: (Year):	7,760: (06)	7940: (07)	:( )
Primary Purpose: Upgrade Existing System(Major)		Percent Trucks: (Year):	:()	:()	:( )
		Projected ADT (HDO): Year		` '	
Please provide a clear pro	oblem statement for this p	project:			
		from the Bluegrass Parky			
		the Nelson County line t			
	rtical Alignment Adequa	acy values are both 3. Th	he roadway has	two 11' lanes wi	ith very
narrow shoulders.					

# **Section III – Project Description** Project Description Narrative:

PIF Revised: Aug. 2004

Reconstruction from Nelson Co Line to Cartwright Creek.

Regional Goals/Objectives Addressed: III-Preserve, maintain, and enhance the existing transportation system to ensure safe, efficient, and effective mobility.

UPL #: 04 115 B0150 121.00 County: Washington Co. #: 115 Route: US-150

**Section IV – Project Area Information:** 

1.	Miscellaneous	A C (1	Existing: Permit	M-4:- T	Existing:	Width: <u>0'</u>
	Roadway Conditions	Access Control:	Proposed:	Median Type:	Proposed:	Width:
	Conditions	Lane	Existing: 2/11'		Existing: Asphalt	Width: <u>1'</u>
		No./Width:	Duan agad. 2/12!	Shoulders:	Dramagad, Asphalt	Width: <b>10'</b>
			Proposed: <u>2/12'</u> Existing: <u>2</u>	Other	Proposed: Asphalt	<del></del>
		No. of Bridges:		Improvement	None □SYP □Resurfa □Other <u>UPL</u>	ice
		Comments:	Proposed: 2	Projects in Area:	Zonici <u>ore</u>	
		Comments:				
2.	Right of Way	Avg.	S MIII	c	r:	
		Width: Exist	ing: 120' Source: HI	S Plans III	licrofilm Other	
		Current Primary U	se:	cial Residential	☐ Farmland ☐ Other:	
		☐ No ⊠ Yes	Project may require additional R/W.	. Possible Reloca	ations: Homes: Busines	ses:
		Comments:	-3			
	********					
3.	Utilities		⊠Power □Gas ⊠	Telephone	Cable Sewer Water	□ITS
		Existing Utilities:	None Other:			_
		☐ No ⊠ Yes	Project may require Utility Relocati	ions. Comments:		
	Environmental	(Check all that apply	):			
	Impacts	⊠Blueline Stream	s ⊠Wetlands ⊠Flo	oodplain	ildlife Managed Areas Histo	oric Properties
		Cemeteries	☐Schools ☐Ch	nurches	ndangered Species Publ	ic Land/Park
		☐Noise Impact	☐ Arch. Sites ☐ NF	R Properties Po	tential NR Properties Othe	r:
		Potential Conta	minated sites:	Landfills		ards Other
		Comments:		<del></del>	_	<del></del>
5.	Air Quality	⊠No □Yes	Project is located in a Maintenance	or Nonattainment A	rea	☐ PM 2.5
		⊠No □Yes	Project adds through lane capacity			
		⊠No □Yes	Project results from a Congestion M	Ianagement Plan		
		⊠No □Yes	Project is included in TIP/STIP		TIP Page # STIP Page	#
		Comments:				
6.	Economic	□No ⊠Yes	Planning/Zoning Regulations	□No ⊠'		
	Impacts	□ No □ Voo	exist in Community  This project has economic impacts.	on regional/least	Commercial or Industria	al Districts.
		☐ No ⊠ Yes	This project has economic impacts  ☐ Development ☐ Tax Revenues ☐ I			
			Please Describe:			
		☐ No ⊠ Yes	This project provides direct access			
				JHistoric Sites □Am	usement Parks US Public Land	Other
			Please Describe: My Old Kentucky Ho			
		☐ No ⊠ Yes	This project provides direct access t  Shopping Centers Schools ☐Ind			
			Please Describe:			

UPL #:	04 115 B0150 121.00			
	County: Washington	Co. #:	115	Route: US-150

				L					
7. Multimodal Opportunities	This project is a candid	late for: (ch	eck all that apply)		cycle Pat		Sidewalks N/A	☐ Shared-Use	e Paths
	This project improves	direct acces	ss to: (check all that a		ports cking Ro		Railways N/A	Riverports	
	Type of Public Transpo	ortation ava	ailable:	☐ Fix	ed Route	e 🖂	Demand Respo	nse	
	Comments:								
8. Social Impacts	This project may affec (Check all that apply)  Comments/Impact Des		Neighborhood or C Travel Patterns (Ve Household Relocat Elderly, disabled, n No adverse effects	chicular, comions ondrivers, m	muter, bi	low-inc			
	Comments/impact Des	criptions.							
Section V – Cost  Cost Estimate by P	Estimate Informatio	n (to be co	mpleted by Hwy Dist	trict Office):					
Phase	Original Estimate	By:	Revision 1	Date	By	:	<b>Revision 2</b>	Date	By:
Planning	\$250,000	JH							
Design	\$2,500,000	JH							
ROW	\$2,000,000	JH							
Utilities	\$1,000,000	JH							
Construction	\$23,000,000	JH							
Total Cost	28,750,000	JH							
Estimate Procedur									
Origina	l Estimate:		Revision	1:			Revi	ision 2:	
Per Mile@	\$		Per Mile@\$	_			Per Mile@	\$	
Terrain:			Terrain:	_			Terrain:		
Detailed Est Calculations			Detailed Estimate Calculations Attac				Detailed Est Calculations		
Estimate Assumption See Rev. 1		-Projec should climbir improv	e Assumptions: et should widen la ers, provide turn ng lanes where no e alignment.	lanes and t	ruck		ate Assumptio		
Estimate Class: E-Rec	quires further study	Estimate	e Class:			Estima	ate Class:		
Section VI – Attac									
The following items	are attached to this doo	cument:	□ Location Map     □	∑ Photog	graph(s)	∐Otł	ner:		
Comments:									

Page 3 of 3 Filename: 04 115 B0150 121^00

# Appendix C – Collision Data

INVESTIGAT	COUNTY_NAM	ROADWAY_NU	ROADWAY_NA	ROADV ROAD	LATITUDE1	LONGITUDE1	MILEPOINT_	COLLISION1	TIME	INTERSECT1	INTEF UNITS_INVO	MOTOR_VEHI	KILLE	D INJUR	RED WEATHER	ROADWAY_	(DIRECTION1	MANNER_OF1	LIGHT_CON1
BARDSTOWN POLICE DEPARTMENT	090	US0150	SPRINGFIELD	RD E	37.76310000	-85.34870000	7.4730	3/22/2009	0335		1		1	0	0 CLEAR	DRY	COLLISION WITH FIXED OBJECT NON - INTERSECTION - FI	RS' SINGLE VEHICLE	DARK-HWY NOT LIGHTED
NELSON COUNTY SHERIFF DEPT.	090	US0150	SPRINGFIELD	RD	37.76320000	-85.34740000	7.5420	12/21/2007	0208		2		2	0	1 RAINING	WET	SIDESWIPE COLLISION - OPPOSITE DIRECTION	SIDESWIPE-OPPOSITE DIRECTION	DARK-HWY NOT LIGHTED
NELSON COUNTY SHERIFF DEPT.	090	US0150	SPRINGFIELD	RD	37.76320000	-85.34710000	7.5580	4/7/2009	1514		1		1	0	0 CLOUDY	DRY	COLLISION WITH FIXED OBJECT NON - INTERSECTION - FI	RS' SINGLE VEHICLE	DAYLIGHT
NELSON COUNTY SHERIFF DEPT.	090	US0150	SPRINGFIELD	RD	37.76320000	-85.34530000	7.6650	6/21/2009	1440		2		2	0	0 CLEAR	DRY	REAR END IN TRAFFIC ONE VEHICLE STOPPED	REAR END	DAYLIGHT
KY STATE POLICE, POST 15	115	US0150	BARDSTOWN	RD	37.76320000	-85.34480000	0.0190	11/25/2009	2018		2		2	0	2 CLOUDY	DRY	1 VEHICLE ENTERING/LEAVING ENTRANCE	ANGLE	DARK-HWY NOT LIGHTED
WASHINGTON CTY SHERIFF DEPT	115	US0150	BARDSTOWN	RD	37.76320000	-85.34430000	0.0500	12/20/2008	1810	CROAKES STATION	RD 2		2	0	0 CLOUDY	DRY	OPPOSING LEFT TURN	OPPOSING LEFT TURN	DARK-HWY NOT LIGHTED
WASHINGTON CTY SHERIFF DEPT	115	US0150	BARDSTOWN	RD	37.76320000	-85.34430000	0.0500	12/4/2009	1910		2		2	0	0 CLEAR	DRY	1 VEHICLE ENTERING/LEAVING ENTRANCE	ANGLE	DARK-HWY NOT LIGHTED
KY STATE POLICE, POST 15	115	US0150	BARDSTOWN	RD	37.76330000	-85.34370000	0.0870	11/11/2009	1600		1		1	0	0 CLOUDY	DRY	OCCUPANT FELL FROM MOVING VEHICLE	SINGLE VEHICLE	DAYLIGHT
KY STATE POLICE, POST 15	115	US0150	BARDSTOWN	RD	37.76320000	-85.34310000	0.1210	4/20/2009	1619	CONNOR	RD 2		2	0	0 CLEAR	DRY	REAR END - OTHER	REAR END	DAYLIGHT
WASHINGTON CTY SHERIFF DEPT	115	US0150	BARDSTOWN	RD	37.76320000	-85.34300000	0.1240	11/20/2009	1954		2		2	0	0 CLEAR	DRY	1 VEHICLE ENTERING/LEAVING ENTRANCE	ANGLE	DARK-HWY NOT LIGHTED
WASHINGTON CTY SHERIFF DEPT	115	US0150	BARDSTOWN	RD	37.76330000	-85.34300000	0.1260	5/16/2009	2159	CONNOR	RD 2		2	0	0 CLEAR	DRY	ANGLE COLLISION - ONE VEHICLE TURNING LEFT	ANGLE	DARK-HWY NOT LIGHTED

### **Crash Calculations for 0.3 mile Spots**

County:	Nelson/Washington
Route:	US150
Period:	6/30/2007 to 5/31/2010

The procedure used below is from The Kentucky Transportation Center, University of Kentucky, College of Engineering, Research Report KTC-09-16/KSP2--09-1F titled "Analysis of Traffic Crash Data in Kentucky (2004-2008).

MV = Million Vehicles = (AADT)\*(No. of Years)\*(365 days/yr.)(10^6)

Functional Class Rate (See table Below)

RC = Critical Accident Rate = (Functional Class Rate) + K\*sqrt((Functional Class Rate)/(MV)) + 1/(2\*(MV))

Total Accident Rate = Total Number of Accidents

MVM

 $\begin{cal} \textbf{Critical Rate Factor} = \underline{Total\ Accident\ Rate} \\ RC \end{cal}$ 

**INPUT** 

Number of Years = 3

K = 2.576

#### \*\*\*Functional Class Rates are for 2004 thru 2008\*\*\*

Functional Cl	ass Rate Ta	ble
3-Year	Period	
	Rural Acc.	Urban Acc.
Highway Type	Rates	Rates
One-Lane	0.74	
Two-Lane	0.64	0.94
Three-Lane	0.42	1.44
Four-Lane Divided	0.32	0.88
Four-Lane Undivided	0.69	1.48
Interstate	0.15	0.3
Parkway	0.18	0.31
All	0.44	0.82

Note: Crash rates are in terms of crashes per million vehicles.

INPUT					OUTPUT			
Begin	End	AADT	Functional	Total No.	MV	RC	Total	Critical
Milepoint	Milepoint		Class Rate	Accidents			Acc. Rate	Rate Factor
	Washington							
Nelson Co.	Co.							
7.512	23.2	8430	0.64	10	9.23	1.37	1.1	0.79

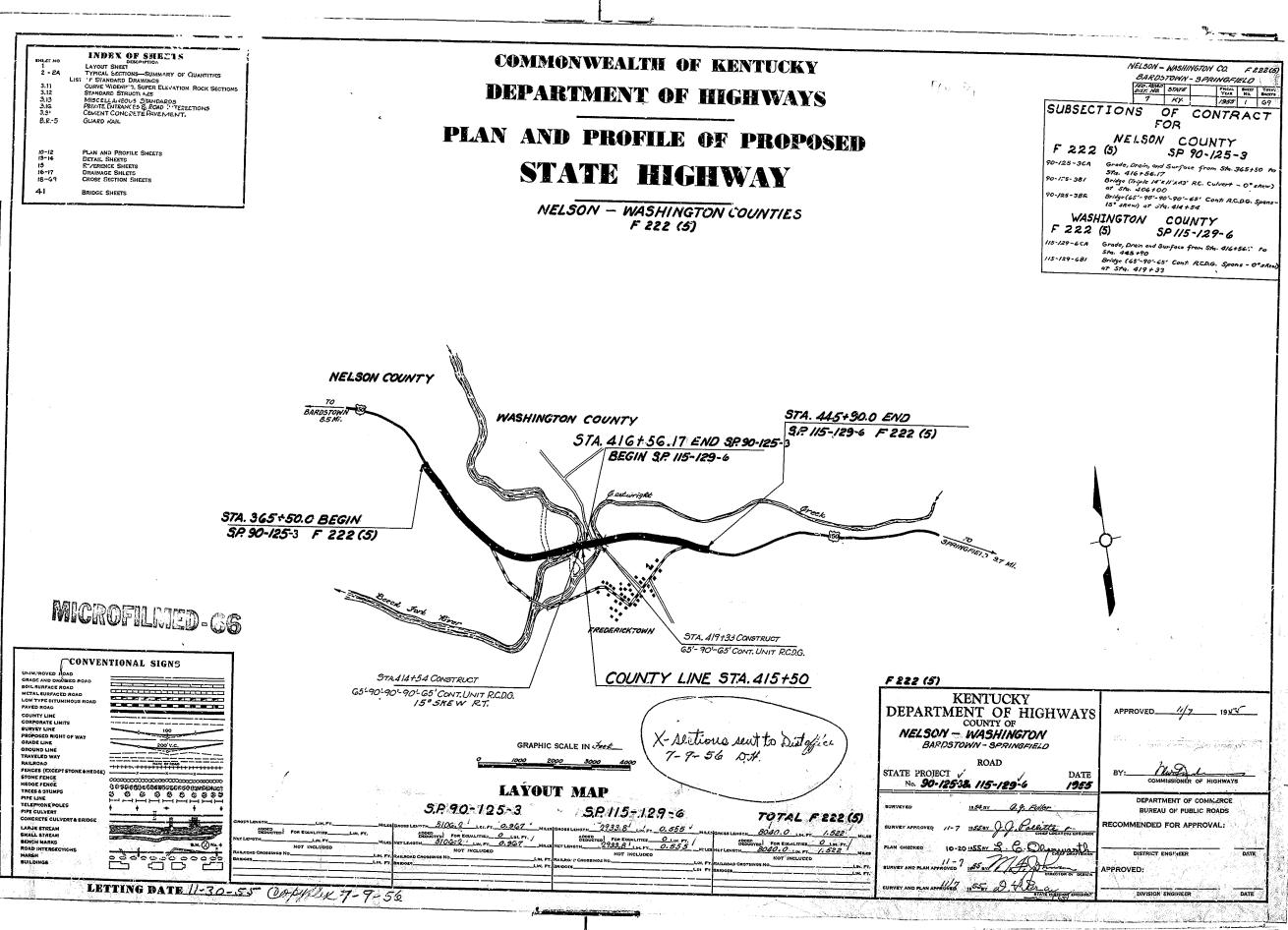
# Appendix D – KYTC's Common Geometric Practices for Rural Arterial Roads

# COMMON GEOMETRIC PRACTICES RURAL ARTERIAL ROADS (OTHER THAN FREEWAYS)

							TRAFFIC	VOLUM	E			
			UN	NDER 400 A.D.T.	)	400-1 A.D.			00-2000 A.D.T.		OVER 2 A.D.1	
	DESIGN SPE	ED 6	40-	-50 M.P.H		40-70 N	1.P.H.	40-	70 M.P.H		40-70 M	.P.H.
	40 MPH 45 MPH			22		22			22			
PAVEMENT WIDTH	50 MPH 55 MPH			22		22					24	
(FEET)	60 MPH 65 MPH 70 MPH			24		24			24			
MINIMUM GRADED 5 SHOULDER WIDTH (FT)	ALL SPEEDS			4		6			6		8	
MINIMUM CLEAR ROADWAY WIDTH OF NEW AND RECONSTRUCTED BRIDGES	ALL SPEEDS					APPR	OACH RO	DADWAY	WIDTH			
	DESIGN SPE	ED		eMAX.				X. 6%		•	MAX. 8%	ò
	30 MPH			300	)		:	275			250	
	35 MPH			420				380			350	
	40 MPH			565			;	510			465	
MINIMUM RADIUS	45 MPH			730	)		(	660			600	
(FEET)	50 MPH			930	)			835			760	
(1221)	55 MPH			1190	)		10	065			965	
	60 MPH			1505	5		1:	340			1205	
	65 MPH			_			1	660			1485	
	70 MPH			_			2	050			1820	
NORMAL PAVEMENT 3 CROSS SLOPES				RA	TE OF C	ROSS SL	.OPE = 2°	%				
NORMAL SHOULDER CROSS SLOPES		EART	H = 8%						PAVED =	: 4%		
MAXIMUM	M.P.H.	30	35	40	45	50	55	60	65	70	75	80
GRADE	LEVEL ROLLING		<u>.                                    </u>		5 6		<u>4</u> 5			3 4		
(PERCENT)	MOUNTAIN		<u> </u>	8		<del>`</del> 7		6	Ι		5	
MINIMUM STOPPING 1	(FEET)	200	250	305	360	425	495	570	645	730	820	910
MINIMUM PASSING SIGHT DISTANCE 2	(FEET)	1090	1280	1470	1625	1835	1985	2135	2285	2480	2580	2680

- MINIMUM STOPPING SIGHT DISTANCES ARE BASED ON HEIGHT OF EYE OF 3.5 FT AND HEIGHT OF OBJECT OF 2.0FT. BOTH HORIZONTAL AND VERTICAL ALIGNMENTS ARE CONSIDERED.
- (2) MINIMUM PASSING SIGHT DISTANCES ARE BASED ON HEIGHT OF EYE 3.5 FT AND HEIGHT OF OBJECT OF 3.5 FT. BOTH HORIZONTAL AND VERTICAL ALIGNMENTS ARE CONSIDERED.
- (3) NORMAL PAVEMENT CROSS SLOPES ON BRIDGES SHALL BE 2%.
- FOR GUIDANCE ON FREEWAYS, REFER TO AASHTO, "A POLICY ON GEOMETRIC DESIGN OF HIGHWAYS AND STREETS", CURRENT EDITION.
- (5) WIDEN 3 FT FOR GUARDRAIL.
- 6 JUSTIFICATION FOR A DESIGN SPEED LESS THAN THE REGULATORY OR POSTED SPEED MUST BE DOCUMENTED AND AVAILABLE FOR REVIEW IN THE PROJECT FILES.

# Appendix E – Existing Roadway Plans



# TYPICAL SECTION

2"Insulation -

7"SubBase

Prime(Tack)\_

1/2"Binder

3"Base-

FISCAL SHEET TOTAL SHEET: 7 KY.

BASIS FOR

CLASS "I"

ESTIMATE

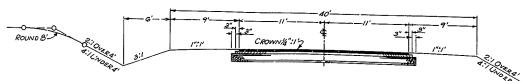
LIMESTONE V

POUNDS PER SQ. YD.

126

151 303

COURSE



NEW CONSTRUCTION: Grade, Drain, and Surface,

BITUMINOUS CONCRETE SURFACE CLASS I [Cumpacted Crushed Limestone Size No. G and No. 10 (50% each). Compacted Crushed Limestone Size No. 2 Waterbound. Construct in two 31/2" courses. 30% (of 7° compacted) additional Limestone screenings Size No.10. . Bituminous Concrete Base. C.10 Gal. per Sq.Yd. Asphalt Cement Q'AC-5. Apply by fogging. Bituminous Concrete Binder, Type B. Prime(Tack)\_ O.10 Gal.per Sq.Yd. Asphall Cement P.A.C-5. Apply by fogging.

11/4"

Prime (Tack) between Base and Binder and between Binder and Surface to be used if directed on construction. , Asphalt Cement P.A.C.-5 shall be used in all Class "I" Mixtures.

The 2"insulation course shall be laid in two applications. The first application shall be Size No.G stone and the second Size No.IO. The course shall be shaped to the cross-section of the pavement. The insulation course shall be watered and compacted in conformity with the specifications for water bound macadam Article 3.7.3. On all but the final rolling of the acreenings the Contractor may use a pneumatic roller of a type approved by the Engineer.

1/4 Surface -Bituminous Concrete Surface, Type B.

Fine aggregate used in Class'I" Bituminous Concrete Surface Type B shall be natural sand the eting the requirements of Article 7.3.4-D-1 of the Departments 1945 Standard Specifications.

Side forms may be eliminated on all base courses except the first course, provided results salisfactory to the Engineer are obtained. On courses where side forms are not used the base stone shall be spread with an approved type self-propelled mechanical stone spreader, and a 2 minimum width earth shoulder placed firmly against the losse stone prior to the initial rolling.

# SUPERELEVATED SECTION

Standard longitudinal metal joints with ties will be required.

Subgrade paper will not be required.

All joints shall be filled with hot noured rubber or a cold applied mastic type crack and joint filler compound.

Shoulder-	Surface———	Shoulder 4
Standard Superelevation	Standard Superelevation	3'-1
Except not flatter than I":!"		1,51

NOTE: Warp shoulder to meet bridge floors.

#### GENERAL SUMMARY

												<i>17</i> -1/\	1													
To	CLEARING AND Grubbing	2000	57	ATION RUCTU COMMON	RE	Overhaul.		REMOVING STONE MASONRY	PIPE		G PROJECT MONUMEN	RIGHT OF WAY	() WATER	FINAL DRESSING	CONC	RSTE CLASS	STEEL	② <sub>BEAM</sub> TYPE	STRUCTURAL STEEL	REMOVAL OF	SEEDING & PROTECTION WITH BLOWN-ON BITUMINOUS TREATED	TURAL	12-12-12 GRANULATED	<b>⊕</b> £	NTRANCE PIPE	 E.
VIT TO BID ON	ACRE		CU. YD.	COMMON	ROCK	YO.STA.	EACH	Cu.Yo.	<u> </u>		<del></del>	<u> </u>			"A"	"D"	REINFOR- CEMENT	GUARD RAIL		STRUCTURE	STRAW MULCH.	GROUND LIMESTONE	COMMERCIAL FERTILIZER	15"	18"	Τ
					SP 9	0-12		CO. 10.		v.FT.	EA	24	100 GAL.	100'STA.	Cu.	Yo.	LB.	Lin.FT.	LB.	LUMPSUM	5a. Yo.	TON	TON		LIN.FT.	=
365+50 to 400+50	13.51	73308		ı <del></del>	0.7. 7	1/567	<del></del>	/2	T - 51	<del></del>			<u> </u>								~					7
400+30 to 415 +56.17	7.04	18460		<del></del>		224006		- /2	56	<u> </u>	<del>                                     </del>	22			QP"								T		32	一
DED FOR CONTINGENCIES		7500			· · · · · · · · · · · · · · · · · · ·			<del></del>	<del> </del>	├──		10	<b>↓</b>		,9°							<del>                                     </del>				+
M DRAINAGE SUMMARIES		101	305	900	130	,		<del>                                     </del>	<del>                                     </del>	<del>}</del> -	1		ļ	<u> </u>	1,0							T				+
TAL NELSON CO.	20.55	<b>3</b> .	. 305	900	/30	235573	1/	12	56	24	1 17		<del>                                     </del>		1595.2		302451	1	2/755			i.			1	+
,			, v			11.5	100	1 /2	136	24	1 1 2	132	4032	1 47.0	/595.2	37.7	302451	7350	2/755		528/5	23.8	4.8		132	I
415+56.17. 420+44.5	4.02	15204			<u> </u>	59833																				
420+445 to 445 t90	8.33	4/31				527614	<del></del>	<del>  </del> -	<del> </del>	<b> </b>		5				T				T T T			$\overline{}$			〒
ED FOR CONTINGENCIES		1500				22/014		ļ	3		1											<b></b>	<del></del>	24		+
M DRAINAGE SUMMARIES		17	5	500	150			<del> </del>	<del> </del>	*	ļ							-				<b>—</b> —	$\vdash$			-
AL WASHINGTON CO.	/2.35	③ i	5	500	150	587447			1 3	<u> </u>					7/7.9	22.4			11743	1						上
TAL F 222(3)	32.90	<b>3</b>	3/0	1400		823020	,		5	<u> </u>	1 7	22		27./	7/7.9	224			11743	/	36480	16.4	3.3	24	19 Jayrey	
Estimated lOGallon perCubic						0.00.20	2	1 /2	59	24	] 3	54	5678	74.1	2313.1	60.1	437929	11250	33498	0 1	89295	40.2	8.1	24	32	4.5

excavation used in embankment.

@Beam Type Guard Rail shall be installed where directed on construction; in conformity with the requirem-ents of Standard Drawing B.R.-5.

See Surfacing Quantities for Roadway Excavation, and note that a quantities have been adjusted to reflect the difference in design thickness of the two types

@Alternate types of Entrance Pipe. Reinforced Concrete, Standard Strength. Corrugated Metal.

## CULVERT PIPE SUMMARY

_							,,,	•
ģ		1	1 CULV	ERT	CLASS	EXCAV	ATION	
SHEET NO.	STATION	SKEW	18"	24"	*A" CONCRETE	STRUCTURE UNCLAS- SIFIED.	ROADWAY (DITCH)	REMARKS
UN	17 70 BID ON		LIM	v.FT.	Cu. Yo.	Cv.	Yo.	1
	5.P.	70	- /25		-			1
Г	366100	30"	64	T ===	3./7	46	10	I-STD, I-ELL.
	370+22	30°	64		3./7	30	8	1-STE, I-ELL.
1	375 135	30°		68	4.23	45	18	1-5TD, 1-ELL.
1.	379 +50	30°	64		3./7	32	18	1-STD, 1-ELL.
10	383+50	300	<i>:8</i>		3.17	32	16	1-5TD,1-ELL.
	387+00	300	68		3./7	30		1-STD, I-ELL
	390175	30°	1/2		3./7	70		1-570,1-ELL
$ldsymbol{ld}}}}}}}$	394 100	30°	140		2.98	20		2-STO.
لـــــ	l	Ш	1		1	,		
Тота	L NELSON CO.		580	68	26.23	305	2) 101	
	5.P. 1	15-	129		er e			
12	443+00		48		2.98	3	71	2-5TD.
/2	446+GG			8	2.00	2	-	1-57U.
1								
DTAL.	WASHING TON CO		48	8	4.98	5 2	7	
T07A	L F2226	5)	628	76	31.21			

Olncluded in general summary. DAlternate types of Culvert Pipe,

Reinforced Concrete, Standard Strength. Triple Strength Vitrified Clay. Cast Iron.

# NOTE: TRANSVERSE JOINTS YA" PREMOULDED FILLER TRANSVERSE ARE NOT APPLICABLE WITH RESPECT TO CLASS T'SURFACE. SURFACING SKETCH AT BRIDGES

#### GENERAL

#### NOTES

All curves to be banked and widened according to Standards or as directed. Superelevation for special cases to be authorized by the District Engineer.

The Contractor is not to order material for drainage structures until the quantities have been checked by the Engineer.

for typical sections in solid rock cuts, see Standard Drawings on Sheet No.3.11 of these plans. Drawings for standard warning signs for the protection of traffic will be furnished by the District

Extra compaction in conformity with Art. 2.5.0 of the Departments 1945 Standard Specifications will be required throughout the project. Guard Rail Posts and Cable to be removed and stored on right-of-way as directed by the Engineer.

Low-bearing soils, identified herein as Soils No.1, 2, and 3, shall not be used in the top 10" of subgrade and replaced within the top 12" of subgrade in cut sections shall be removed and replaced with Selected Soils having a C.B.R. value of 8.2 or greater, identified herein as Soils No.4,5,6, and 7.

The Standard Specifications for State and Federal Road and Bridge Construction, edition of 1945, as amended by the amendments, published in Pamphlet No.3 of Approved Provisions, Specifications, and Amendments, with the following Provisions, Special Specifications and Amendments, will apply on this project.

No. 34-R Bituminous Fiber or Bituminous Cark Expansion Joint Filler. » Amendment No. 36 Asphalt Cement. ✓ Amendment No. 39-R Measurement of Bituminous Materials. / Amendment Composition of Mixtures (Bituminous). No. 41 , Amendment No. 42-R Grading Requirements for Aggregates.

Emergency Provision No. 13 Deferment or Cancellation.

(Special Specification No.36-R-2 Membrane Curing. (May be used on Cament)

Special Specification No. 43-R-1 Hot-Poured Rubber-Type Crack and Joint Filler Compound. Special Specification No. 46-R-1 Cold-Applied Mastic-Type Crack and Joint Sealer Compound. Special Specification No. 49-R Grass and Agricultural Seed for Seeding.

Special Specification No. 52 Roadside Improvement. Special Specification No.45-R Surface Finish ExposedConcrete.

EROSION CONTROL NOTES.

I Required Contract Provisions for Federal Aid Primary, Urban, and Interstate Projects. The road may be closed to through traffic.

NELSON CO. & WASHING TON CO. F 222(5) BARDSTOWN-SPRINGFIELD RD.

FISCAL SHEET TOTAL SHEET!

#### FOR SUPFACINIC

	LIN.FT.	SQ. YDS.	MILES
S.P. 90-125			
GROSS LENGTH	5/06.2		
NET LENGTH	37 08.2	<del>-</del>	0.967
404.3 DEDUCTED FOR BRIDGE	4701.9	4	0.890
ADDED FOR CURVE WIDENING AND APPROACH	/Es	849	9 22' 9 22.5' 9 23' 9 24'
TOTAL AREA		12327	@ 22'
NELSON COUNTY	,	12604	@ 23'
S.P. 115-129			
GROSS LENGTH	2933.8		0.555
NET LENGTH 224.0' DEDUCTED FOR BRIDGE	2709.8		0.5/3
ADDED FOR APPROACHES		2685 6 275 2 6 2820 6 29 54 6	9 22' 9 22.5'
TOTAL AREA Washington County		9310 9526 9744 6	9 22.5 9 23'
TOTAL PROJECT F 2	22(5)		
GROSS LENGTH	8040.01		1.522
NET LENGTH G28.3' DEDUCTED FOR BRIDGES	74/1.7	/	1.403
ADDED FOR CURVE WIDENING AND APPROACHE	is	35/96 36016 36846 38486	22.5'
TOTAL AREA F 222 (5)		2/637 6 22/306 226246 236/26	22.5'

### SURFACING

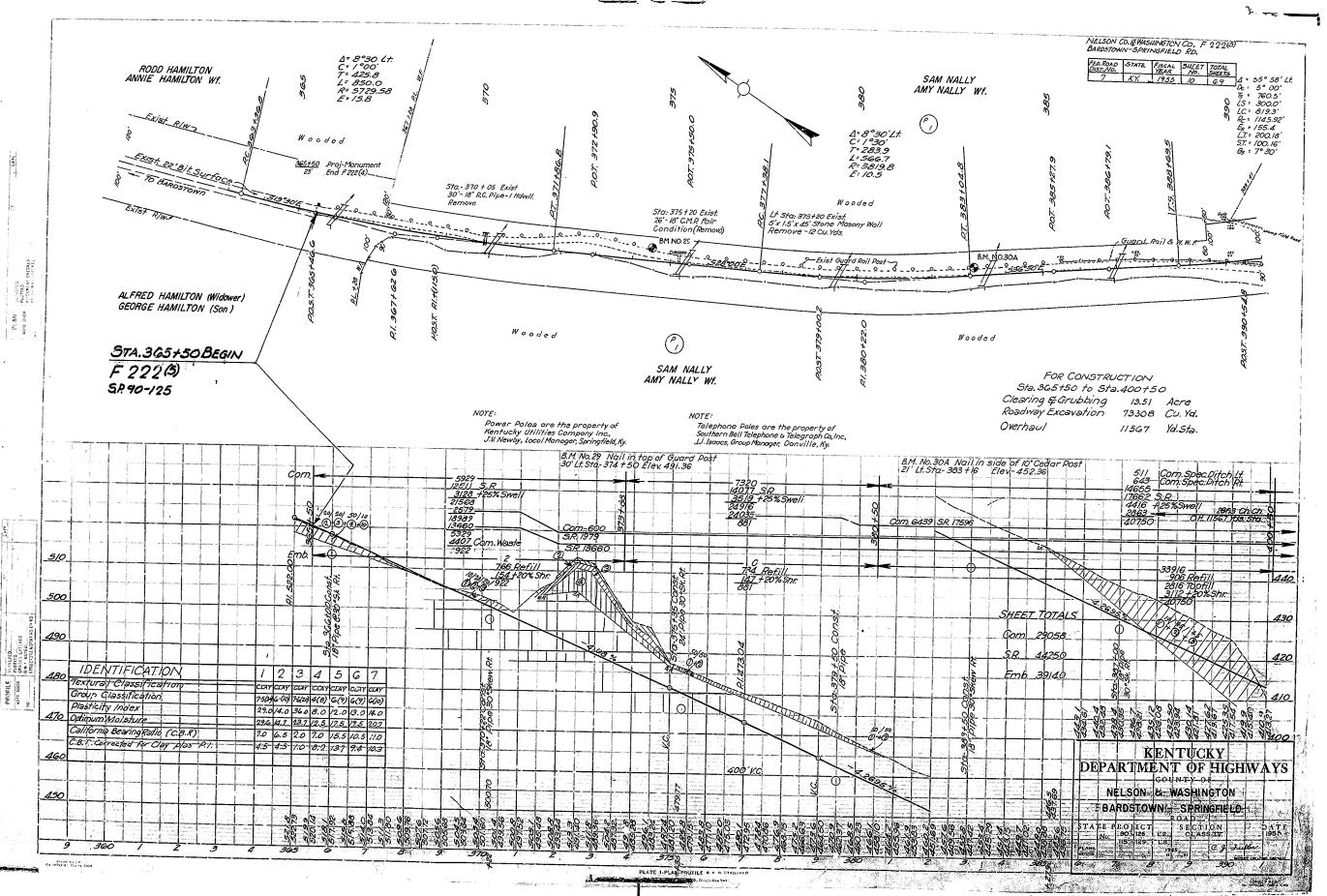
### QUANTITIES

- COA	V / /	///_~	,	
ITEM	UNIT	. S.P. .90~,125	. 5.P.	TOTAL F 222(5
CEMENT CONCRETE PAVEMEN	疔		2	1/2/2/
Crushed Limestone Size No. G for Insulation.	Ton	960	130	1690
Crushed Limestone Size No. 10 for Insulation	Ton	960	730	1 , - , -
8"Uniform Cement Concrete Pavement.	Sq.Yd		9310	
Crushed Limestone Size No. G10 for Entrance& MailTurnaits.	Ton		1510	21637 3) 485
	+		57.	3) 403
Roadway Excavation	CU.YU.	99055	20794	119849
BITUMINOUS CONCRETE SURFA	CE	CLAS		
Crushed Limestone No. 2	Ton	4295/	3245	7540
Crushed Limestone No.6	Ton	G151	465	1080
Crushed Limestone No.10	Ton	19051	1435	3340
Cituminous Concrete Base	Ten	19501	1480	3430
Bituminou s Concrete Binder, Type 8	Ton	9501	720	1670
Bituminous Concrete Surface, Type B	Ton	77.51	1 590	1365
Asphalt Cement, P.A.C5	Gal.	2550	1930	4480
Crushed Limestone Size No.GIO for Entrance & Mail Turnouts.	TON	325		3) <i>485</i>
Roadway Excavation	CUXU.	99800	-20894	120694
Bituminous Conc Binder Type 8 and or Surt. Type 8 (for leveling)	Ten	160	120	280 V
			1.6 (25.01)	No. 7 1./27

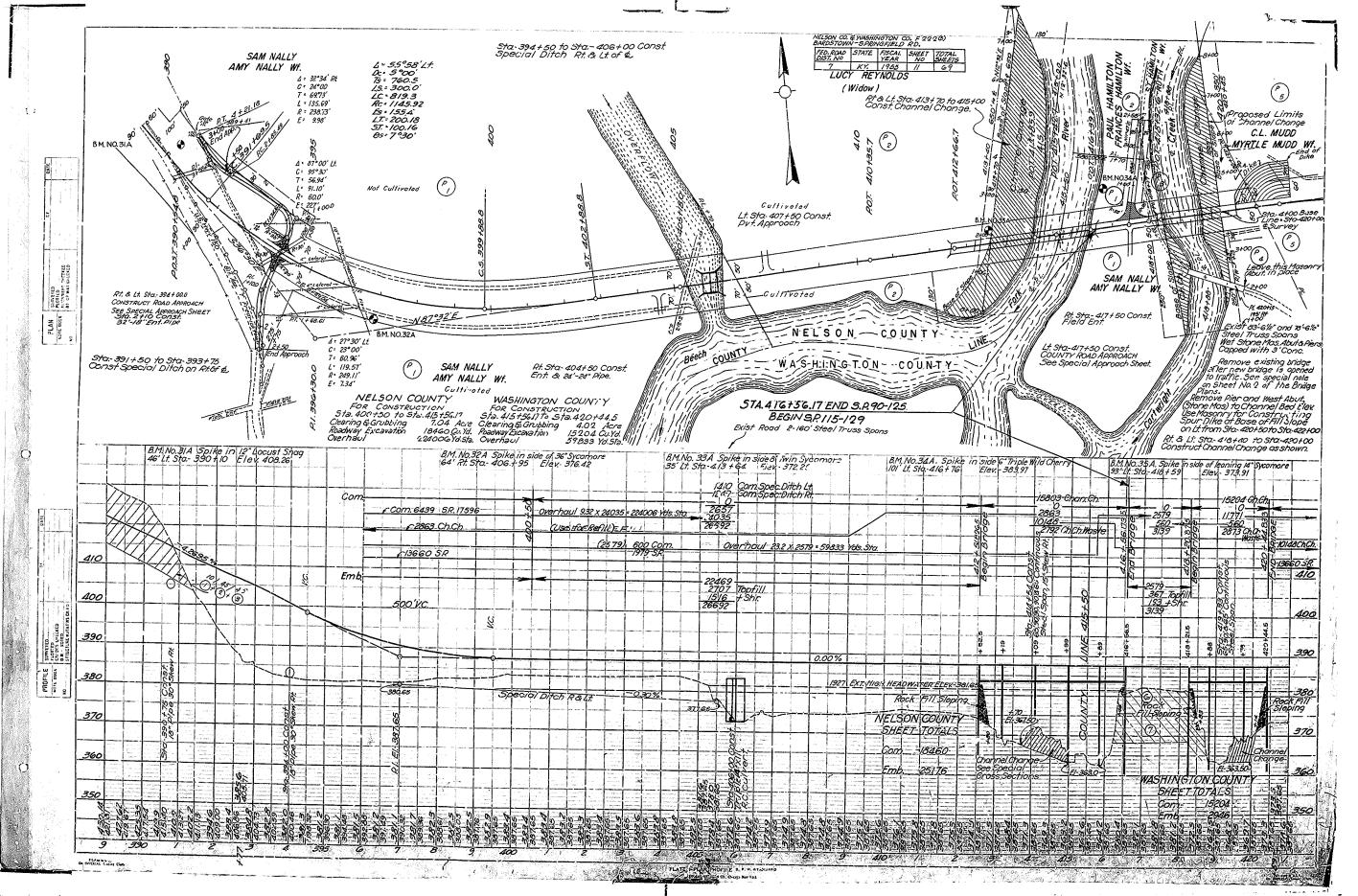
3 Includes 140 tons tor maintenance of local traffic.

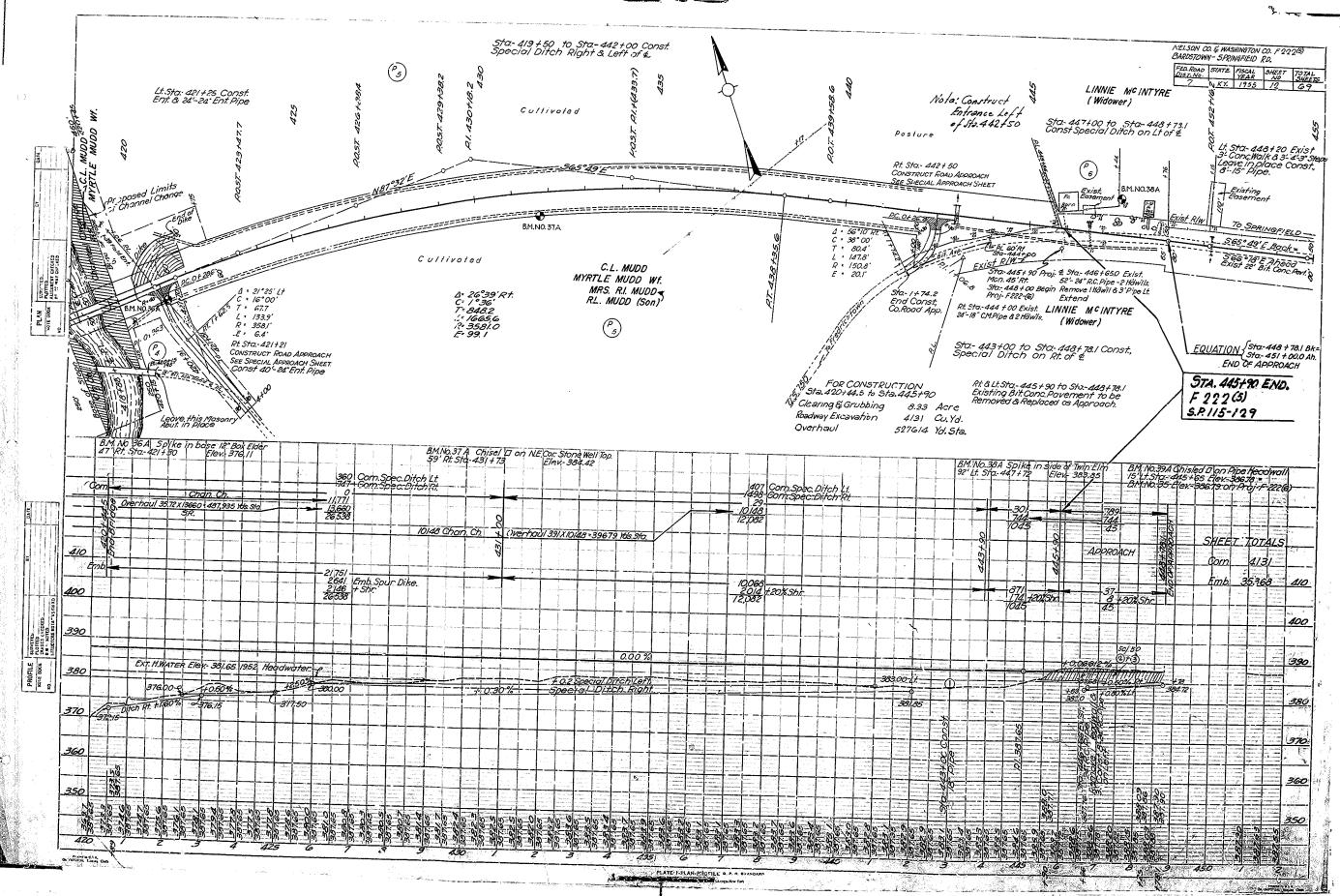
# BRIDGE AND CULVERT SUMMARY

1 1 2 1														1/1/				
STATION	SIZE	Skaw	INLET ELEVATION LENGTH	ELEVATION		GRADE	DRAWING NUMBER	\$ 6 5	FILL ON CULVERT	CLASS	CRETE CLASS *D"	STEEL RENFOR- CEMENT	EXCAL	SOLID	STRUCTURAL STEEL	REMOVE EXISTING STRUCTURE	Existing Structure	CHANNEL CHANGE
UNIT TO BID OF	~								1,0		v. YD.	!===	<u> </u>	ROCK				0,,,,,,,
in the state of th	SA	2 6	70-125						_==	1	J. 1U.	LB.	<i>C</i>	u.YD.	Lb.	LUMPSUM		T
																		 <del></del>
11 3414+54 G5'-	14'x11'x43'R.C.Culvert.	_	372.5 21'-6"	372.2	21-6"	387.65	12240	Spc1.	3.4'	335.4		47088	10.50	<del></del>				 
Cont	Unit R.C.D.G. Bridge	15					/2238			1/233.6	<u> </u>	255363			10000			See Road Plans
70711 0/21				L					<b>—</b>		<del>                                     </del>	(22000	3775	1750	21755			See Road Plans
TOTAL NELSO	N Co.							===		£)/569.0	(4) 000	()						
Beer dilly	5.F	5.1	15-109					===		0,367.0	3.7	<u> 4)302451</u>	£ 900	(d) 130	2/755			
11 (5)419+33 63	6-00/00/		<del></del>	- 1				_					£					
Cont	Unit RC.D.G. Bridge	Ш					12239	Spc/.	ī	7/2.9	22.4	/35478	Fool	18.1				
								<del>-</del>	1	77.2.7	22.4	1334780	5001	150.	11743	/	83'-62.5'-78'-675" TRUSS SPANS WETSTONE MAS. PIERS & ABUTS.	 SeeRoad Plans
TOTAL WASHING	S'FON								_	4 200 0								 -
TOTAL F 2	2 2 (5)	V .			====					d) 712.9	e) 22.4	D 135478	4) 50 <u>0</u>	4) 150	11743	1		
										D 2281.9	1 601	137029	1400	D 280 (	£33498	<b>a</b> (1		 
Sucioded in Ge	eneral summar	y.	<b>⑤</b>	Rock fill	s to be	made a	round a	hutm	ente			101727	,,,,,,	200	- 334 78	/		 •
				Rock fill indicat	ed in .	distribut	ion on	plan	sheer	S No. 11	and No. 1	oridge pi z.	ans an	ď				5.00

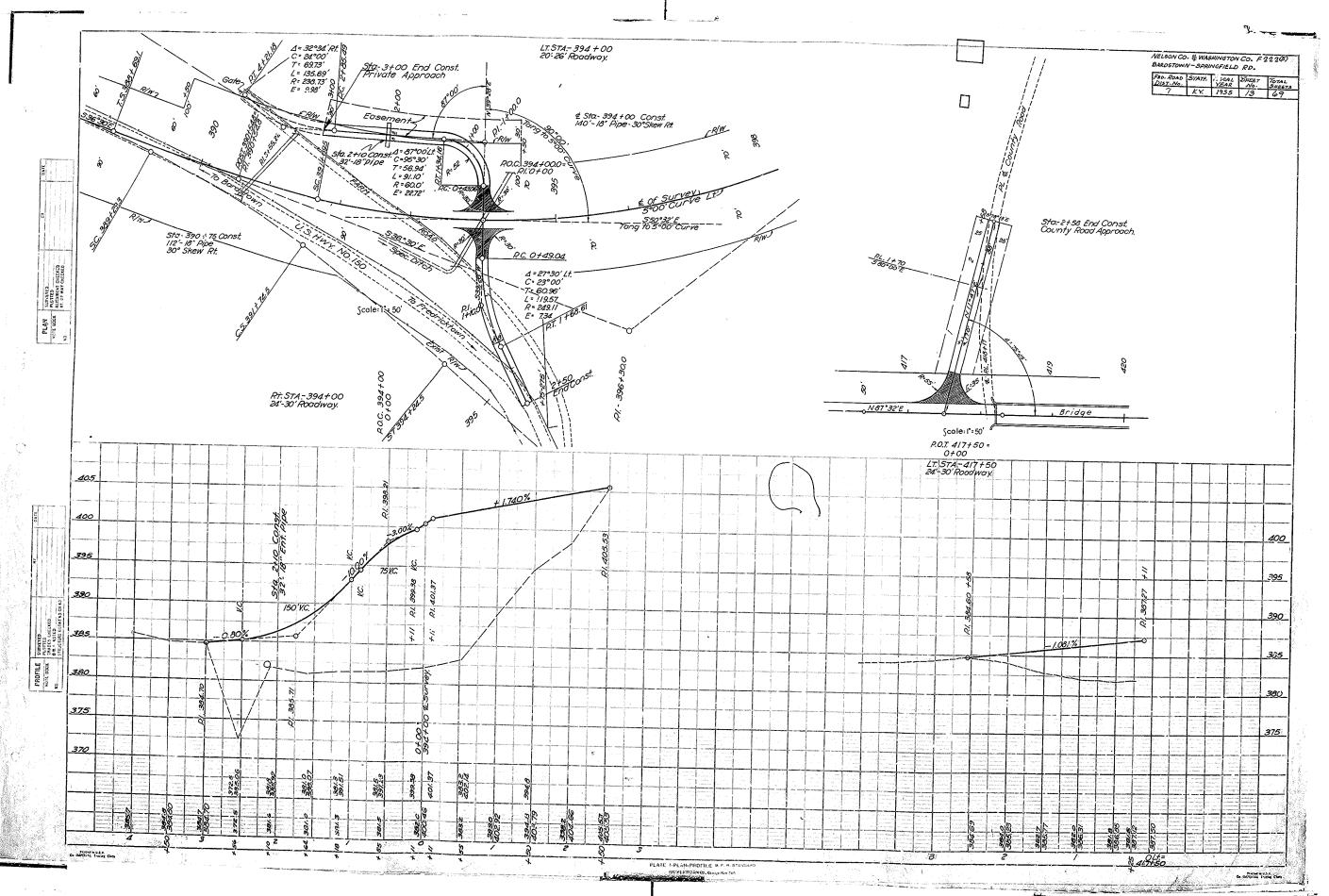


SP 90 - 125 - 3

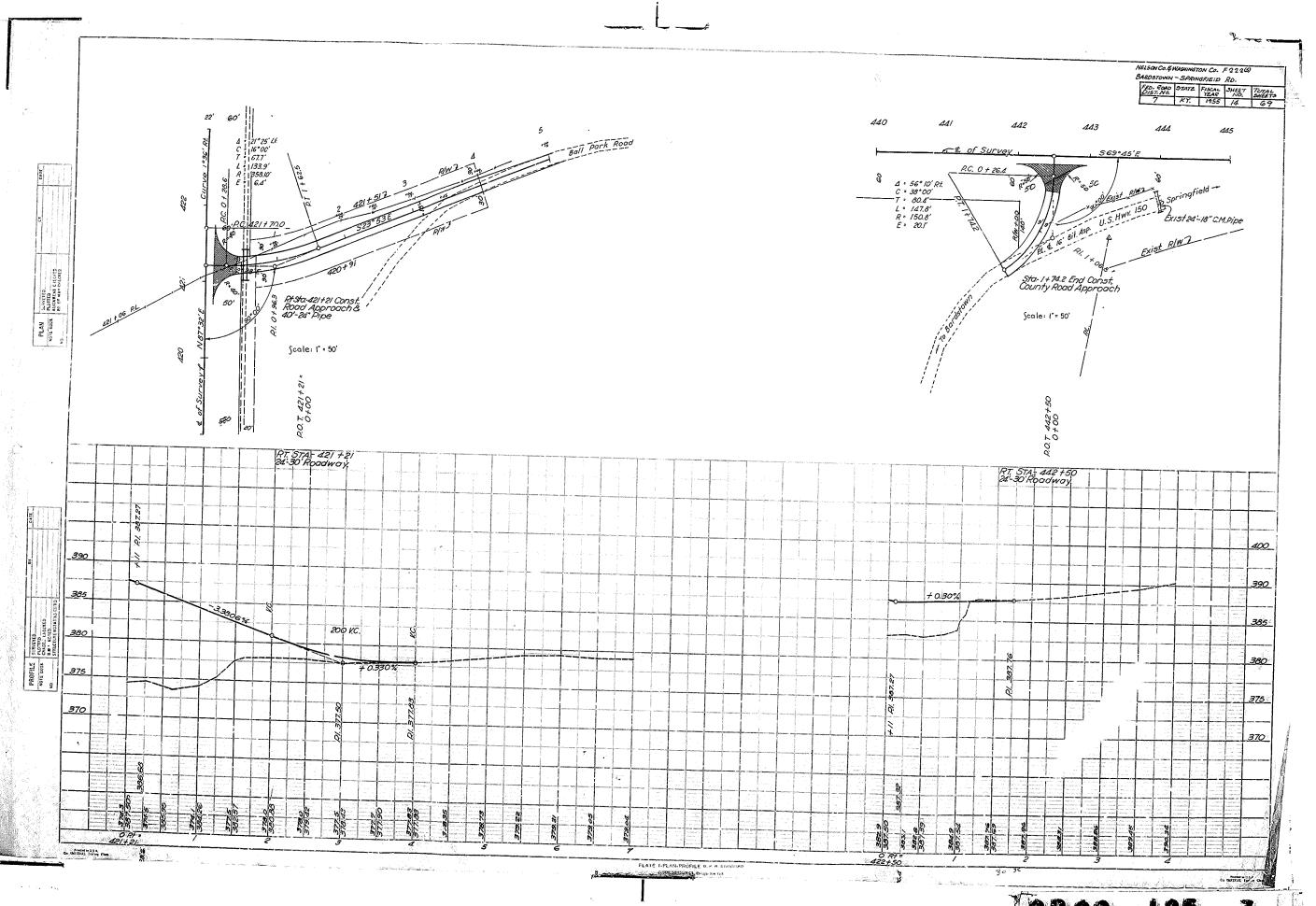




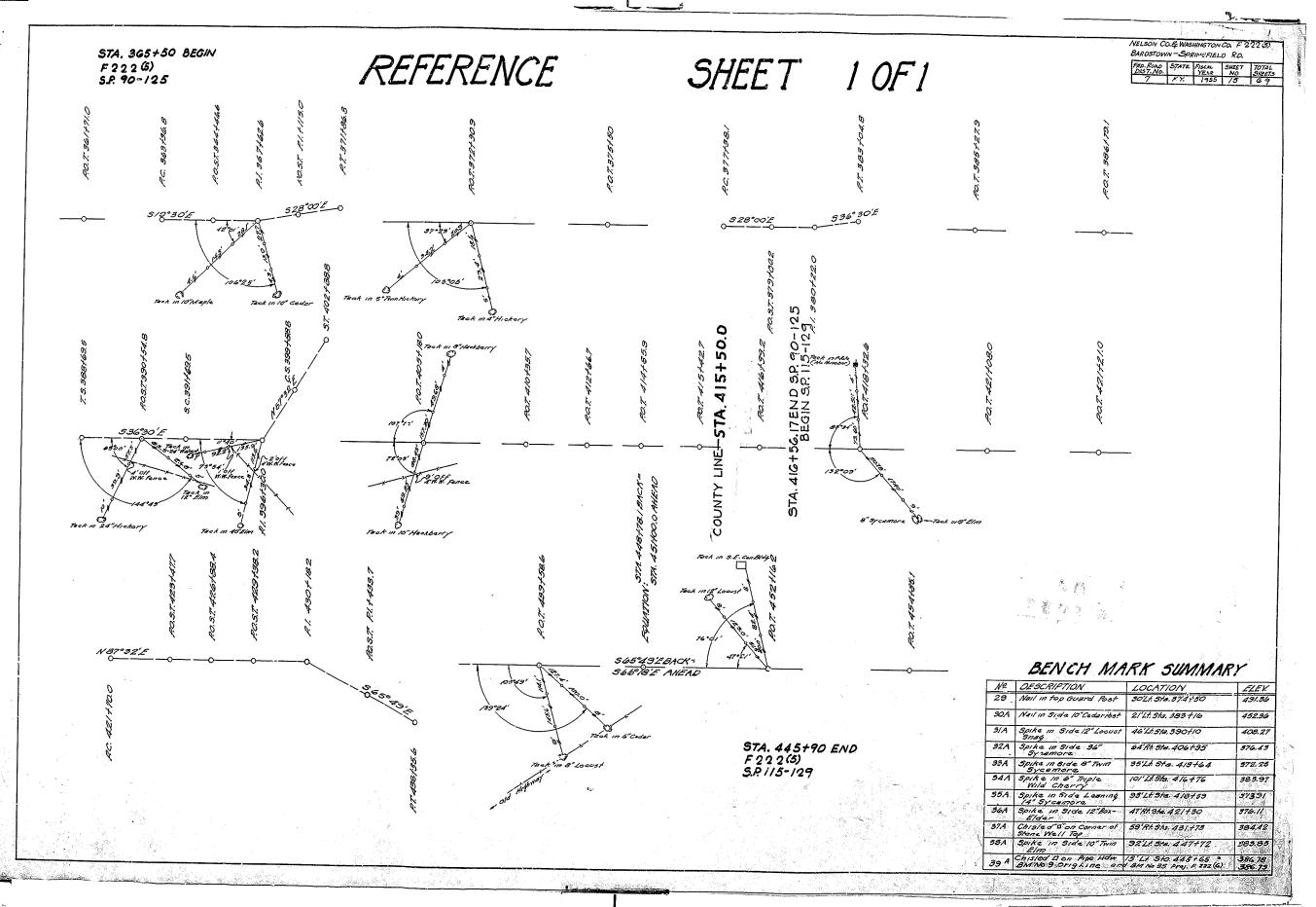
8P 90 - /25 - 3



SP90-125-3



SP 90 - 125 - 3



# Appendix F – Structure Inventory and Appraisal Sheets

Bridge Key: 10843 Agency ID: 090B00028N SR: 45.5 SD/FO: SD

		LION

State 1: Struc Num 8: 090B00028N

Facility Carried 7: Location 9: ON WASHINGTON -NELSON CL

Rte.(On/Under)5A: Route On Structure 2 U.S. Numbered Rte. Signing Prefix 5B:

Rte. Number 5D:

00150

Directional Suffix 5E: 0 N/A (NBI) % Responsibility: Unknown SHD District 2: District 4 County Code 3: Nelson (090) Place Code 4: FIPS 0000 Mile Post 11: 7.656 mi

Feature Intersected 6: BEECH FORK

Longitude 17: 085d 20' 43"

Border Bridge Code 98:

Border Bridge Number 99:

Level of Service 5C:

#### STRUCTURE TYPE AND MATERIALS

Number of Approach Spans 46: 0 Number of Spans Main Unit 45: 5

Main Span Material/Design 43A/B:

2 Concrete Continuous 04 Tee Beam

Deck Type 107: 1 Concrete-Cast-in-Place 3 Latex Concrete/Similar Wearing Surface 108A:

0 None Membrane 108B: Deck Protection 108C:

#### AGE AND SERVICE

1957 Year Reconstructed 106: 0

Type of Service on 42A: 1 Highway Type of Service under 42B: 5 Waterway

Lanes on 28A: 2 Lanes Under 28B: 0 Detour Length 19: 8.1 mi ADT 29: Truck ADT 109: % Year of ADT 30: 2009

#### GEOMETRIC DATA

Length Max Span 48: 89.9 ft Structure Length 49: Curb/Sdwlk Width L 50A: 2.5 ft Curb/Sidewalk Width R 50B: 2.5 ft Width Curb to Curb 51: 27.9 ft Width Out to Out 52: 33.1 ft Approach Roadway Width 32: 25.9 ft (w/ shoulders) Median 33: 0 No median

Deck Area: 13,415.5 sq. ft

Skew 34: 15.00 ° Structure Flared 35: Vertical Clearance 10: 99.99 ft Horiz. Clearance 47: 27.89 ft

Minimum Vertical Clearance Over Bridge 53: 328.1 ft

Minimum Vertical Underclearance Reference 54A: N Feature not hwy or RR

Minimum Vertical Underclearance 54B:

Minimum Lateral Underclearance Reference R 55A: N Feature not hwy or RR

Minimum Lateral Underclearance R 55: Minimum Lateral Underclearance L 56:

#### INSPECTION

Frequency 91: 2/22/2010 Next Inspection: 02/22/2012 FC Frequency 92A: NA FC Inspection Date 93A: NA Next FC Inspection: NA UW Frequency 92B: NA UW Inspection Date 93B: NA Next UW Inspection: NA

SI Frequency 92C: NA SI Date 93C: Next SI:

Element Frequency: 24 months Element Inspection Date: 02/22/2010 Next Elem. Insp. Due: 02/22/2012

#### CLASSIFICATION

Defense Highway 100: 0 Not a STRAHNET hwy Parallel Structure 101: No || bridge exists Direction of Traffic 102: 2 2-way traffic Temporary Structure 103: Not Applicable (P) Highway System 104: 0 Not on NHS NBIS Length 112: Long Enough Toll Facility 20: 3 On free road Functional Class 26: 06 Rural Minor Arterial Defense Hwy 110: 0 Historical Significance 37: 5 Not eligible for NRHP

Owner 22: 01 State Highway Agency

Custodian 21: 01 State Highway Agency

#### CONDITION

6 Satisfactory Super 59: 4 Poor Sub 60: 6 Satisfactory Culvert 62: N N/A (NBI) Channel/Channel Protection 61: 7 Minor Damage

#### LOAD RATING AND POSTING

Inventory Rating Method 65: 2 AS Allowable Stress Operating Rating Method 63: 2 AS Allowable Stress

Inventory Rating 66: Operating Rating 64:

Design Load 31: 4 M 18 (H 20) Posting 70: 5 At/Above Legal Loads

Posting status 41: A Open, no restriction

#### **APPRAISAL**

Approach Rail 36C: Bridge Rail 36A: 0 Substandard 1 Meets Standards 1 Meets Standards Approach Rail Ends 36D: 1 Meets Standards Transition 36B: Deck Geometry 68: Str. Evaluation 67: 4 Tolerable

Underclearance, Vertical and Horizontal 69: N Not applicable (NBI)

Approach Alignment 72: 6 Equal Min Criteria Waterway Adequacy 71: 7 Above Minimum

Scour Critical 113: 4 Stable, needs action

#### PROPOSED IMPROVEMENTS

Bridge Cost 94: \$ 0 Type of Work 75: Unknown (P) \$0 Length of Improvement 76: 0.0 ft Roadway Cost 95: Future ADT 114: Year of Cost Estimate 97: Unknown Year of Future ADT 115: 2029

#### **NAVIGATION DATA**

Navigation Control 38: 0 0

Vertical Clearance 39: 0.0 ft Horizontal Clearance 40: 0.0 ft Pier Protection 111: 1 Not Required Lift Bridge Vertical Clearance 116:

#### **ELEMENT CONDITION STATE DATA**

Str Unit	Flm/Fnv	Description	Units	Total Qtv	% in 1	Qtv. St. 1	% in 2	Qty. St. 2	% in 3	Oty St 3	% in 4	Oty St 4	% in 5	Oty St 5
Oli Oliil	LIIII/LIIV	Description	Office	Total Qty	/0 111 1	Qty. Ot. 1	70 III Z	Gty. Ot. 2	70 111 3	Qty. Ot. 3	70 III <del>4</del>	Qty. Ot. 4	70 111 3	Qty. Ot. 3
1	18/1	P Conc Deck/Thin Ovl	(SF)	11,969	0 %	0	100 %	11,969	0 %	0	0 %	0	0 %	o
1	110/1	R/Conc Open Girder	(LF)	1,612	0 %	0	0 %	0	99 %	1,600	1 %	12	0 %	0
1	205/1	R/Conc Column	(EA)	12	100 %	12	0 %	0	0 %	0	0 %	0	0 %	0
1	215/1	R/Conc Abutment	(LF)	75	0 %	0	76 %	57	24 %	18	0 %	0	0 %	0
1	234/1	R/Conc Cap	(LF)	141	100 %	141	0 %	0	0 %	0	0 %	0	0 %	0
1	302/1	Compressn Joint Seal	(LF)	59	100 %	59	0 %	0	0 %	0	0 %	0	0 %	0

Str Unit	Elm/Env	Description	Units	Total Qty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4	% in 5	Qty. St. 5
1	311/1	Moveable Bearing	(EA)	20	0 %	0	60 %	12	40 %	8	0 %	0	0 %	0
1	313/1	Fixed Bearing	(EA)	4	100 %	4	0 %	0	0 %	0	0 %	0	0 %	0
1	331/1	Conc Bridge Railing	(LF)	806	1 %	6	74 %	595	25 %	205	0 %	0	0 %	0
1	359/1	Soffit Smart Flag	(EA)	1	0 %	0	100 %	1	0 %	0	0 %	0	0 %	0
1	361/1	Scour Smart Flag	(EA)	1	0 %	0	100 %	1	0 %	0	0 %	0	0 %	0
1	363/1	Section Loss SmFlag	(EA)	1	0 %	0	100 %	1	0 %	0	0 %	0	0 %	0
1	503/1	RC Curb	(LF)	806	7 %	56	50 %	400	43 %	350	0 %	O	0 %	0
1	505/1	RC Sidewalk	(LF)	806	26 %	206	74 %	600	0 %	0	0 %	0	0 %	0
1	606/1	Drains	(EA)	1	100 %	1	0 %	0	0 %	0	0 %	O	0 %	0
Str Unit	Elm/Env	Description			·		Ele	ment Note	S					
1	18/1	Concrete Deck - Protected w/ Thin	< none	9 >										
1	110/1	Reinforced Conc Open Girder/Bea	Concr	ete beams	have be	en retofitte	d with a	void filling	material	, High Stre	ngth Ste	el Sheets,		
			steel v	and Coati	applied	on each gi	rder. Gir	der 2 & 3 s	span 1 ha	ardwire is	debondin	g in small		
				from the boire is debo							. Girder	3 span 3,		
1	205/1	Reinforced Conc Column or Pile E			namy m	oman area	<u> </u>	ic bottom c	or the gir	4010.				
1	215/1	Reinforced Conc Abutment	Abutm	ents have	minor to	moderate	cracking	with leach	ning and	minor spa	lls.			
1	234/1	Reinforced Conc Cap	< none	∋ >										
1	302/1	Compression Joint Seal	< none	∋ >										
1	311/1	Moveable Bearing (roller, sliding, e	Rocke	rs at Abutr	ment 1 a	re slightly e	expande	d. Bearings	s at abut	ments hav	e minor s	ection los	s.	
1	313/1	Fixed Bearing												
1	331/1	Reinforced Conc Bridge Railing	Concr	Concrete railing have moderate cracking, scaling, and minor spalls.										
1	359/1	Soffit of Concrete Deck or Slab	Deck underside has minor cracking with leaching. Span 1 and span 5 has hardwire placed on soffit near abutment 1 and abutment 5.											
1	361/1	Scour		Moderate sour at piers 2, 3, and 4.										
1	363/1	Section Loss	Minor	section los	s at the	abutment l	oearings.							
1	503/1	Reinforced Concrete Curb	Curbs	have mod	erate cra	acking, sca	ling and	minor spal	ls.					
1	505/1	Reinforced Concrete Sidewalk	Sidew	idewalk has minor cracking and scaling.										
1	606/1	Drains	< none	9 >										

#### **BRIDGE NOTES**

All of the repairs made to the girders will maintain the weight capacity at the current level before the repairs were made. Crack gauges were installed on this structure where vertical cracks were repaired on the girders. Diaphragms over piers 4 & 5 have hardwire applied to them.

PAST INSPECTION	١	
Inspection Date:	02/22/2010	Type: 2 Standard (24 months)
Inspector:	TLAWLER	Pontis User Key: TLAWLER - Todd I
Scope: NBI: Underwater	Ü Other:	Element: Ü
INSPECTION NOT	ES	
PAST INSPECTION		
Inspection Date:	03/12/2008	Type: 2 Standard (24 months)
Inspector:	EHARDIN	Pontis User Key: EHARDIN - Ernest
Scope: NBI: Underwater	Ü Other: : Fracture Critical:	Element: Ü
INSPECTION NOT	ES	

PAST INSPECTION	N	
Inspection Date:	02/01/2006	Type: 2 Standard (24 months)
Inspector:	DKEMPER	Pontis User Key: DKEMPER - David
Scope: NBI: Underwater	Ü Other:	Element:
INSPECTION NOT	ES	

**INSPECTOR WORK CANDIDATES** 

Next Inspection:

03/03/2012

### Structure Inventory and Appraisal Sheet (English Units)

Bridge Key: 13486 Agency ID: 115B00022N SR: 40.8 SD/FO: SD

Frequency 91

SI Frequency 92C: NA

IDENTIFICATION

Struc Num 8: 115B00022N State 1: 21 Kentucky

.1 MI E OF NELSON CL Facility Carried 7: US-150 Location 9:

Rte. Signing Prelix 5B: 2 U.S. Numbered Hwy Rte.(On/Under)5A: Route On Structure

Level of Service 5C: 1 Mainline Rte. Number 5D: 00150 Directional Suffix 5E: 0 N/A (NBI) % Responsibility: Unknown SHD District 2: Washington (115) District 4 County Code 3: Place Code 4: FIPS 0000 Mile Post 11: 0.085 mi

Feature Intersected 6: CARTWRIGHT CREEK

Latitude 16: 37d 45' 48" Longitude 17: 085d 20' 37°

Border Bridge Code 98: Unknown (P)

Border Bridge Number 99:

STRUCTURE TYPE AND MATERIALS

Number of Approach Spans 46: 0 Number of Spans Main Unit 45: 3

Main Span Material/Design 43A/B:

2 Concrete Continuous 04 Tee Ream

Deck Type 107: 1 Concrete-Cast-in-Place

Wearing Surface 1084: 3 Latex Concrete/Similar Membrane 108B: 0 None

Deck Protection 108C: None

AGE AND SERVICE

Year Built 27: Year Reconstructed 106: 0

Type of Service on 42A: 1 Highway Type of Service under 42B: 5 Waterway

anes on 28A; 2 Lanes Under 28B: 0 Detour Length 19: 8.7 mi ADT 29: 8.290 Truck ADT 109: % Year of ADT 30: 2009

**GEOMETRIC DATA** 

Length Max Span 48: 89.9 ft Structure Length 49: 225.1 ft Curb/Sdwlk Width L 50A: 2.6 ft Curb/Sidewalk Width R 508: 2.6 ft Width Curb to Curb 51: 27.6 ft Width Out to Out 52: 30.5 ft Approach Roadway Width 32: 25.9 ft (w/ shoulders) Median 33: 0 No median

Deck Area: 6,867.2 sq. ft

Skew 34: 0.00 ° Structure Flared 35: 0 No flare Vertical Clearance 10: 99.99 ft Horiz, Clearance 47: 27.56 ft

Minimum Vertical Clearance Over Bridge 53: 328.1 (1

Minimum Vertical Underclearance Reference 54A: N Feature not hwy or RR

Minimum Vertical Underclearance 54B:

Minimum Lateral Underclearance Reference R 55A: N Feature not hwy or RR

Minimum Lateral Underclearance R 55: 0.0 ft Minimum Lateral Underclearance L 56: 0.0 (1 INSPECTION

24 months Inspection Date 90:

SI Date 93C:

FC Frequency 92A: NA FC Inspection Date 93A: NA Next FC Inspection: NA UW Frequency 92B: NA UW Inspection Date 93B: NA Next UW Inspection: NA

3/3/2010

Element Frequency: 24 months Element Inspection Date: 03/03/2010 Next Elem. Insp. Due: 03/03/2012

CLASSIFICATION

Defense Highway 100: 0 Not a STRAHNET hwy Parallel Structure 101: No || bridge exists Temporary Structure 103: Not Applicable (P) Direction of Traffic 102: 2 2-way traffic NBIS Length 112: Highway System 104: 0 Not on NHS Long Enough Toll Facility 20: 3 On free road Functional Class 26: 06 Rural Minor Arterial Historical Significance 37: 5 Not eligible for NRHP Defense Hwv 110:

Owner 22: 01 State Highway Agency Custodian 21: 01 State Highway Agency

CONDITION

Ueck 58: 5 Fair Super 59: 4 Poor Sub 60: 6 Satisfactory 7 Minor Damage

Culvert 62: N N/A (NBI)

Channel/Channel Protection 61:

LOAD RATING AND POSTING

Inventory Rating Method 65: 2 AS Allowable Stres Operating Rating Method 63: 2 AS Allowable Stress

Inventory Rating 66: HS22.2 Operating Rating 64: HS22.2

Design Load 31: 4 M 18 (H 20) Posting 70: 5 At/Above Legal Loads

Posting status 41: A Open, no restriction

**APPRAISAL** 

0 Substandard Approach Rail 36C: 1 Meets Standards Bridge Rail 36A: 1 Meets Standards Approach Rail Ends 36D: 1 Meets Standards Transition 36B: Deck Geometry 68: 3 Intolerable - Correct Str. Evaluation 67:

Underclearance, Vertical and Horizontal 69: N Not applicable (NBI)

Waterway Adequacy 71: 7 Above Minimum Approach Alignment 72:

Scour Critical 113: 8 Stable Above Footing

PROPOSED IMPROVEMENTS

Bridge Cost 94: \$ 861,000 Type of Work 75: 34 Widen w/ Deck Reha Roadway Cost 95: \$0 Length of Improvement 76: 22.6 ft Total Cost 96: \$ 860,000 Future ADT 114: 12 352 Year of Future ADT 115: Year of Cost Estimate 97: 1995 2029

**NAVIGATION DATA** 

0 0 Navigation Control 38:

Horizontal Clearance 40: Vertical Clearance 39: 0.0 ft 0.0 ft Pier Protection 111: 1 Not Required Lift Bridge Vertical Clearance 116:

#### **ELEMENT CONDITION STATE DATA**

Str Unit	Elm/Env	Description	Units	Total Oty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4	% in 5	Qty. St. 5
1	18/1	P Conc Deck/Thin Ovl	(SF)	6,160	0 %	0	100 %	6,160	0 %	0	0 %	0	0 %	0
1	110/1	R/Conc Open Girder	(LF)	880	0 %	0	100 %	875	0 %	4	0 %	0	0 %	0
1	205/1	R/Conc Column	(EA)	6	100 %	6	0 %	0	0 %	0	0 %	0	0 %	0
1	215/1	R/Conc Abutment	(LF)	110	0 %	0	0 %	0	100 %	110	0 %	0	0 %	o
1	234/1	R/Conc Cap	(LF)	70	100 %	70	0 %	a	0 %	0	0 %	0	0%	Q
1	302/1	Compressn Joint Seal	(LF)	66	100 %	66	0 %	d	0 %	0	0 %	0	0%	q

7 Above Min Criteria

Bit Unit   Birt    Description   Units   Total Orly   % in 1   City   City   % in 1   City   City			Otractare mive		<u> </u>	11017	, bb	uio	ui O	1100	- (	9113	,,, O	11163	<i></i>
1 313/1 Fixed Bearing (EA) 100 % 40 % 0 0 0 % 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	_		<del></del>				ity. St. 1	% in 2	Oty. St. 2	% in 3	Oty. St. 3	% in 4	Oty. St. 4	% in 5	Qty. St. 5
1 \$31/1 Conc Bridge Railing (LF) 440 0 % 0 0% 0 100 % 410 0% 0 0% 0 0% 0				(EA)	12	33 %	4	67 %	8	0%	0	0 %	C	0 %	0
1   159/1   Soffit Smart Flag   (EA)   1   0 %   0   100 %   1   0 %   0   0 %   0   0 %   0   0 %   0   0				(EA)	4	100 %	4	0 %	0	0%	0	0 %	0	0%	0
1   S03/1   RC Curb   (LF)   44d   0 %   0   100 %   438   0 %   2   0 %   0   0 %   0   0 %   0   0 %   0   0	-			(LF)	440	0 %	0	0%	0	100 %	440	0 %	0	0%	0
Sir Unit Elm/Env Description				(EA)	1	0 %	0	100 %	1	0 %	0	0 %	0	0 %	O
1   18/1   Concrete Deck - Protected wf Thin   Minor cracking and potholes.     1   10/1   Reinforced Conc Open Girder/Bea Girders have minor to moderate cracking. Repairs have been made to deter any further cracking. Hardwife has been added to the bottoms and sides of all beams in each span. Girder 4 at abutment 4 bearing has a targe spall exposing rebar which has moderate section loss.     1   205/1   Reinforced Conc Column or Pile   Schone >     2   15/1   Reinforced Conc Abutment   Abutments have minor to moderate cracking, spalling, and scaling exposing rebar.     1   302/1   Reinforced Conc Cap	1	503/1	RC Curb	(LF)	440	0 %	0	100 %	438	0 %	2	0 %	0	0 %	0
110/1   Reinforced Conc Open Girder/Beas Girders have minor to moderate cracking. Repairs have been made to deter any further cracking. Hardwire has been added to the bottoms and sides of all beams in each span. Girder 4 at abutment 4 bearing has a large spall excepting reber which has moderate section loss.    205/1   Reinforced Conc Column or Pite Expression   Abutments have minor to moderate cracking, spalling, and scaling exposing reber.     234/1   Reinforced Conc Cap			<u>`</u>					Ele	ment Note	es :					
205/1   Reinforced Conc Column or Pile   Expression   Substitution   Substituti						•									
1   205/1   Reinforced Conc Column or Pile   Conces   Abutment   Abutments have minor to moderate cracking, spalling, and scaling exposing rebar.     1   234/1   Reinforced Conc Cap	1	110/1	Reinforced Conc Open Girder/Bear	lerackir	ıa. Hardwi	re has bee	in added	to the bo	ottoms and	l sides of	all beams	in each s	nan Gird	ler 4	
1   234/1   Reinforced Conc Cap	1	205/1	Reinforced Conc Column or Pile E	< none	>			pan onpo	<u> </u>	PHILIPPIN II	20 11100010	10 3001101	1033.		
1 302/1 Compression Joint Seal 1 311/1 Moveable Bearing (roller, sliding, eAbutment bearings have minor to moderate deterioration with minor to moderate section loss. 1 313/1 Fixed Bearing 1 331/1 Reinforced Conc Bridge Railing Rails have moderate deterioration. 1 359/1 Soffit of Concrete Deck or Stab Deck underside has minor to moderate cracking with leaching. Hardwire has been added to the soffit in spans 1 and 3 from the abutments to 30' out, also added to the pier diaphrams. Curbs have minor to moderate cracking and spalling.  PAST INSPECTION Inspection Date: 03/03/2010 Type: 2 Standard (24 months) Inspector: DKEMPER Pontis User Key: DKEMPER - Davic  Scope: NBI: V Other: Element: V Underwater: Fracture Critical:	1 2	215/1	Reinforced Conc Abutment	Abutm	ents have	minor to m	oderate	cracking	spalling,	and scali	ng exposir	ng rebar.			
1 311/1 Moveable Bearing (roller, sliding, eAbulment bearings have minor to moderate deterioration with minor to moderate section loss.  1 313/1 Fixed Bearing  1 331/1 Reinforced Conc Bridge Railing Rails have moderate deterioration.  1 359/1 Soffit of Concrete Deck or Slab Deck underside has minor to moderate cracking with leaching. Hardwire has been added to the soffit in spans 1 and 3 from the abutments to 30' out, also added to the pier diaphrams.  Curbs have minor to moderate cracking and spalling.  BRIDGE NOTES  PAST INSPECTION  Inspection Date: 03/03/2010 Type: 2 Standard (24 months)  Inspector: DKEMPER Pontis User Key: DKEMPER - Davic  Scope:  NBI: ✓ Other: Element: ✓  Underwater: Fracture Critical:	1	234/1	Reinforced Conc Cap	< none	>										
1 313/1 Fixed Bearing 1 331/1 Reinforced Conc Bridge Railing 1 359/1 Soffit of Concrete Deck or Stab Deck underside has minor to moderate cracking with leaching. Hardwire has been added to the soffit in spans 1 and 3 from the abutments to 30' out, also added to the pier diaphrams.  Curbs have minor to moderate cracking and spalling.  BRIDGE NOTES  PAST INSPECTION Inspection Date: 03/03/2010 Type: 2 Standard (24 months) Inspector: DKEMPER Pontis User Key: DKEMPER - Davic  Scope:  NBI:  Other:	1	302/1	Compression Joint Seal											_	
1 313/1 Fixed Bearing 1 331/1 Reinforced Conc Bridge Railing Rails have moderate deterioration. 1 359/1 Soffit of Concrete Deck or Stab Soffit in spans 1 and 3 from the abutments to 30' out, also added to the pier diaphrams. 1 503/1 Reinforced Concrete Curb Curbs have minor to moderate cracking with leaching. Hardwire has been added to the soffit in spans 1 and 3 from the abutments to 30' out, also added to the pier diaphrams.  BRIDGE NOTES  PAST INSPECTION Inspection Date: 03/03/2010 Type: 2 Standard (24 months) Inspector: DKEMPER Pontis User Key: DKEMPER - Davic  Scope:  NBI:  Other:	1	311/1	Moveable Bearing (roller, sliding, e	Abutm	ent bearing	gs have mi	inor to m	oderate o	deteriorați	on with m	inor to mo	derate se	ction loss		
1 359/1 Soffit of Concrete Deck or Stab Deck underside has minor to moderate cracking with leaching. Hardwire has been added to the soffit in spans 1 and 3 from the abutments to 30' out, also added to the pier diaphrams.  BRIDGE NOTES  PAST INSPECTION Inspection Date: 03/03/2010 Type: 2 Standard (24 months) Inspector: DKEMPER Pontis User Key: DKEMPER - Davic  Scope:  NBI:  Other:  Element:  Underwater:  Fracture Critical:  Underwater:  Fracture Critical:	1					-									
1 359/1 Soffit of Concrete Deck or Stab Deck underside has minor to moderate cracking with leaching. Hardwire has been added to the soffit in spans 1 and 3 from the abutments to 30' out, also added to the pier diaphrams.  Curbs have minor to moderate cracking and spallling.  BRIDGE NOTES  PAST INSPECTION Inspection Date: 03/03/2010 Type: 2 Standard (24 months) Inspector: DKEMPER Pontis User Key: DKEMPER - Davik  Scope:  NBI:  Other:	1	331/1	Reinforced Conc Bridge Railing	Rails h	ave mode	rate deteri	oration.							_	
BRIDGE NOTES  PAST INSPECTION Inspection Date: 03/03/2010 Type: 2 Standard (24 months) Inspector: DKEMPER Pontis User Key: DKEMPER - Davic Scope:  NBI:    Other:    Element:    Underwater:    Fracture Critical:	1			Deck u soffit in	nderside h	nas minor i	o moder	ate crack	ing with le	aching. I	lardwire h	as been a	added to t	he	
PAST INSPECTION Inspection Date: 03/03/2010 Type: 2 Standard (24 months) Inspector: DKEMPER Pontis User Key: DKEMPER - Davic  Scope:  NBI:  Other:  Element:  Underwater:  Fracture Critical:	1	503/1	Reinforced Concrete Curb	Curbs	have mino	r to moder	ate craci	king and	spalling.	100 4440	3 to 0.0 pt	, ондрига			
Inspection Date: 03/03/2010 Type: 2 Standard (24 months)  Inspector: DKEMPER Pontis User Key: DKEMPER - Davic  Scope:  NBI:  Other:  Element:  Underwater: Fracture Critical:															
Inspector: DKEMPER Pontis User Key: DKEMPER - Davic  Scope:  NBI:  Other:  Element:  Underwater:  Fracture Critical:	PASTI	NSPE	CTION												
Scope:  NBI:    Other:    Element:    Underwater:    Fracture Critical:	Inspect	tion Da	te: 03/03/2010		Туре	: 2 Sta	ndard	(24 mc	nths)						
NBI:  Other:  Element:  Underwater:  Fracture Critical:	inspect	tor:	DKEMPER		Ponti	s User I	Key:	DKEM	PER - [	Davic					
Underwater: Fracture Critical:	Scope:														
		NBI:	Other:				Elemer	nt: 📑	7						
INSPECTION NOTES		Under	water: Fracture	Critic	al:										
	INSPE	CTION	NOTES												
			<del></del>												
														963 -	

PAST INSPECTION	N	
Inspection Date:	03/17/2008	Type: 1 SIA (Initial Inventory)
Inspector:	JNOBLIN	Pontis User Key: JNOBLIN - Jim Nc
Scope: NBI: Underwater	Other: Fracture Critical	☐ Element: ✓
INSPECTION NOT	ES	
PAST INSPECTION	V	
Inspection Date:	03/01/2006	Type: 2 Standard (24 months)
Inspector:	DKEMPER	Pontis User Key: DKEMPER - David
Scope: NBI: Underwater	Other: Fracture Critica	Element:
INSPECTION NOT	ES	
INSPECTOR WOR		

# Appendix G – FIRM Map(s) of the Study Area

### **LEGEND**



SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

**ZONE A** No Base Flood Elevation determined.

**ZONE AE** Base Flood Elevations determined.

**ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations

determined.

**ZONE AO** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths

determined. For areas of alluvial fan flooding, velocities also determined.

**ZONE AR** Area of special flood hazard formerly protected from the 1% annual chance flood event

by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual

chance of greater flood event.

**ZONE A99** Areas to be protected from 1% annual chance flood event by a Federal flood protection

system under construction; no Base Flood Elevations determined.

**ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations

determined.

**ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.



#### FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.



#### OTHER FLOOD AREAS

ZONE X A

Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

#### OTHER AREAS

**ZONE X** Areas determined to be outside the 0.2% annual chance floodplain.

**ZONE D** Areas in which flood hazards are undetermined, but possible.

urance Program at 1-800-638-6620.



MAP SCALE 1" = 500"

250 0 250 500 750 1,000

NFIP

ROGRAM

 $\Gamma$ 

FLOOD INSURANG

ATTIONAL

PANEL 0239D

## **FIRM**

FLOOD INSURANCE RATE MAP BELL COUNTY, KENTUCKY AND INCORPORATED AREAS

#### **PANEL 239 OF 360**

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

 COMMUNITY
 NUMBER
 PANE
 SUFFIX

 BELL COUNTY
 210010
 0239
 D

 MIDDLESBORO, CITY OF
 215190
 0239
 D

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

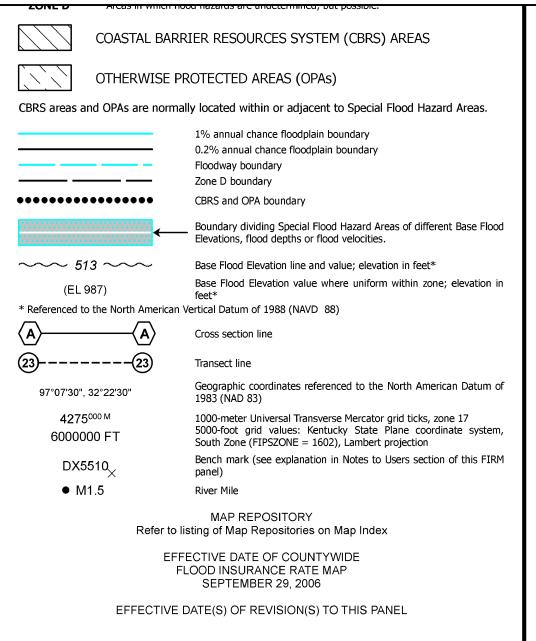


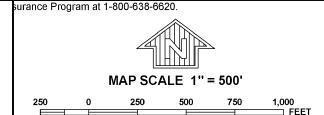
MAP NUMBER 21013C0239D

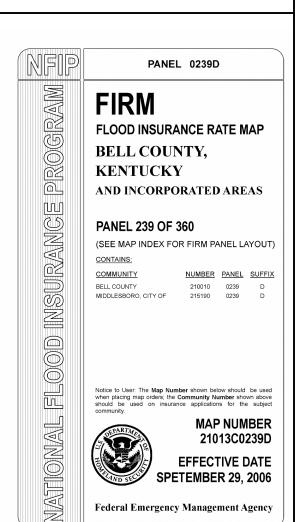
EFFECTIVE DATE SPETEMBER 29, 2006

Federal Emergency Management Agency

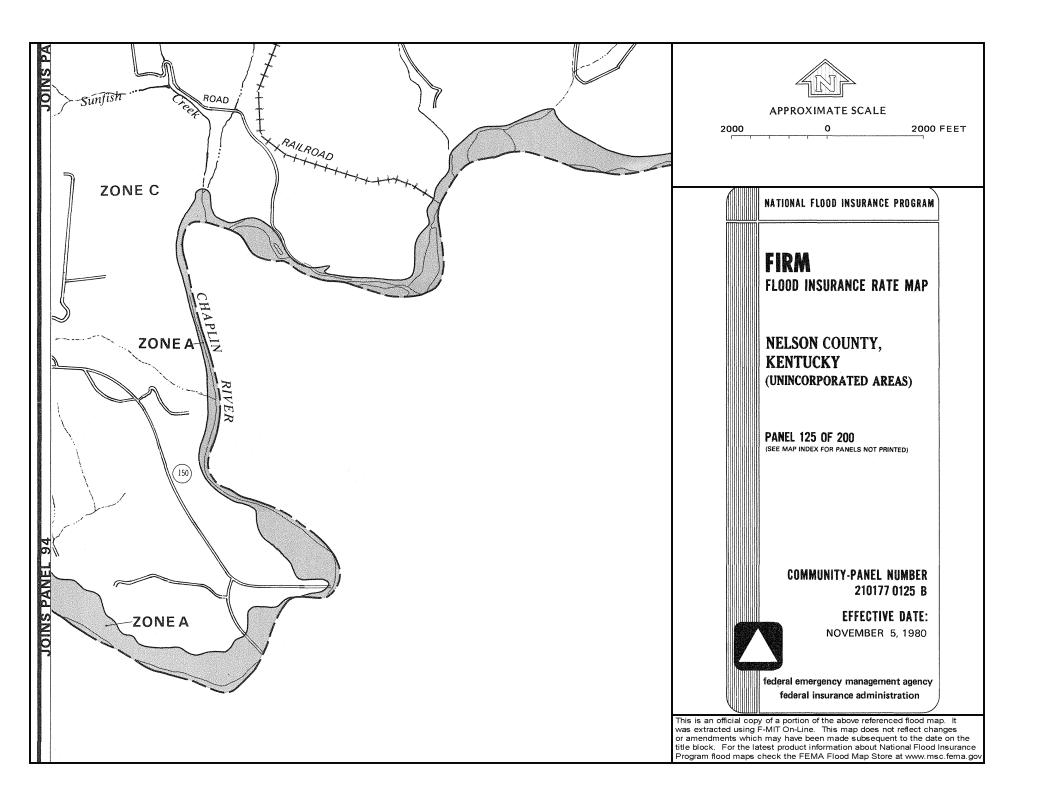
This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov

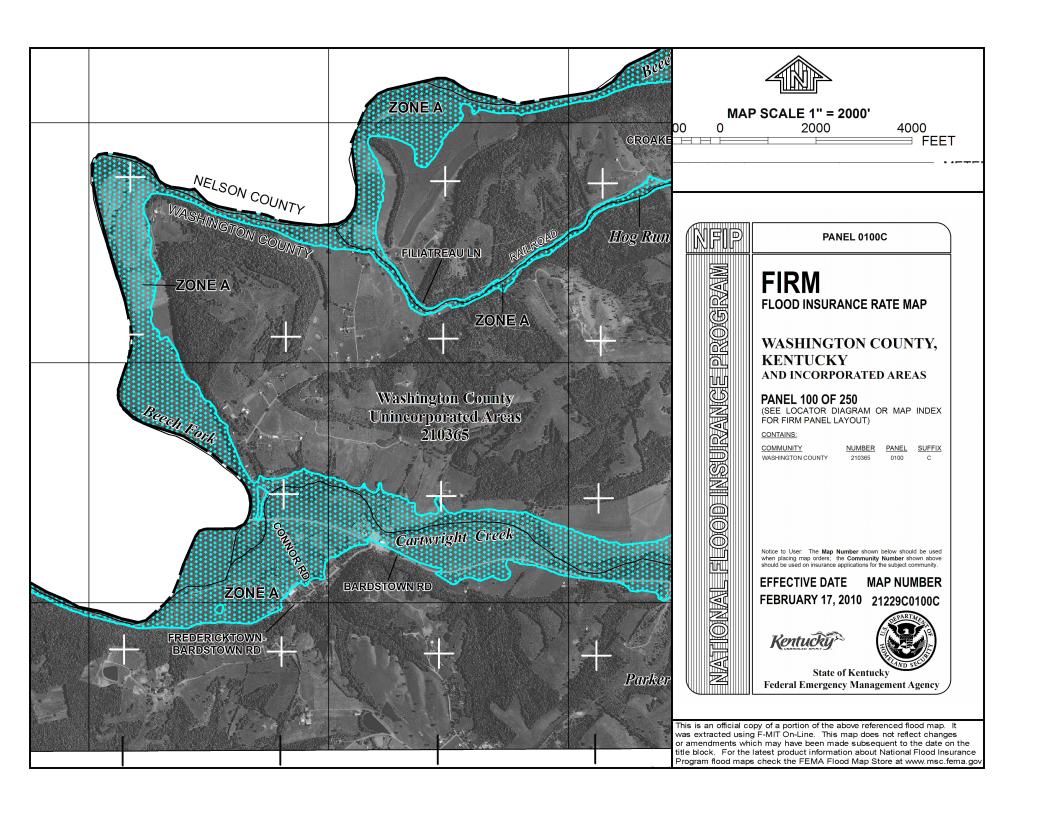






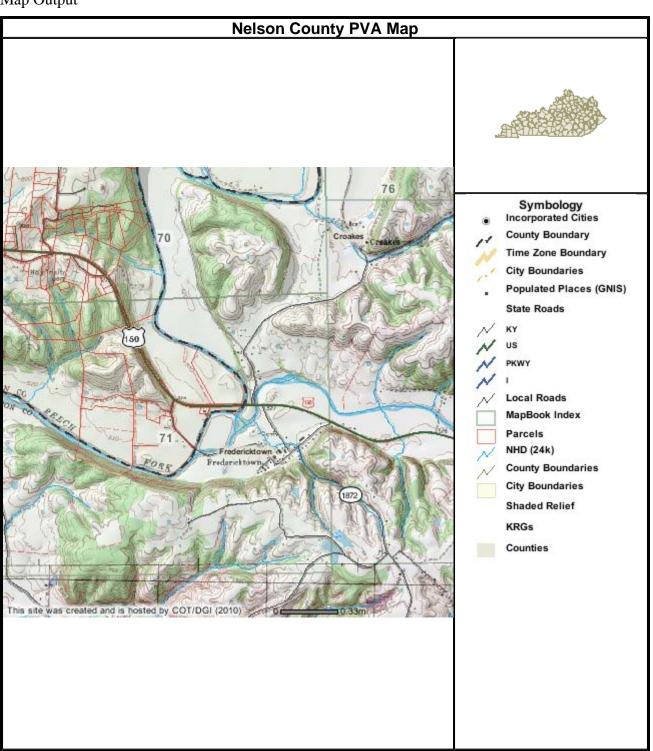
This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov





# Appendix H – Nelson County PVA Map

Map Output Page 1 of 1



# Appendix I – Pictures



Bridge Over Beech Fork West Approach



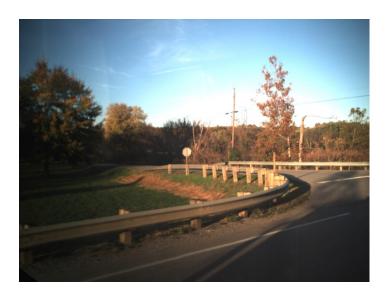
Bridge over Cartwright Creek looking East



Bridge Over Beech Fork East Approach



Intersection with Connor Road



**Croakes Station Road** 



Fredericksburg Park on the Right



Looking left from Croakes Station Road



Cartwright Creek



Collection of Debris (Cartwright Creek)



Bridge over Cartwright Creek



View from Bridge over Cartwright Creek looking West





Debris







Under Bridge over Beech Fork



Bridge Rail



Top of Culvert



Three Span Culvert (Beech Fork Overflow)



Wingwall & Culvert

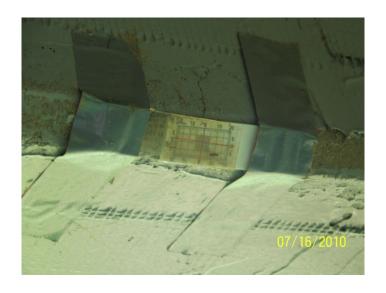


Headwall Separation at Culvert





Headwall at Culvert (Roadway Drainage Problems)



## Appendix J – Project Team Meeting Minutes

## MEETING MINUTES

**Project:** Pre-Design Scoping Study for 4-1068 & 4-1069

**Purpose:** Project Team Meeting

Place: Kentucky Transportation Cabinet (KYTC), District 4 Conference

Room, Elizabethtown, Ky.

**Meeting Date:** July 16, 2010, 9:30 am EST

**In Attendance:** Kevin Young KYTC-D4 Planning

Jared Clemons KYTC-D4 Design/Planning

Josh Hornbeck KYTC-D4 PD&P
Patty Dunaway KYTC-D4 CDE
David Kemper KYTC-D4 Structures

Jude Filiatreau KYTC-D4 PD&P, Bardstown Chad Filiatreau KYTC-D4 PD&P, Bardstown

John Edwards KYTC-D4 Utilities Kevin Blain KYTC-D4 Traffic

Joseph Ferguson KYTC-D4 Environmental

John Moore KYTC-D4 Project Development Brad Eldridge KYTC-CO Highway Design

Charlie Spalding
Sreenu Gutti
Scott Thomson
Jill Asher

KYTC-CO Planning
KYTC-CO Planning
KYTC-CO Planning

**INTRODUCTIONS:** Jill opened the Project Team Meeting by discussing the purpose of the Pre-Design Scoping Studies. These studies, formerly known as First Look Studies, are not new to D4 or some of the other districts. It is anticipated that a study of this type will be done for every project preceding the design phase if there is no planning study associated with the project. The nine elements of Purpose and Need as defined by NEPA will be addressed and used to create a purpose and need statement for each project. Pre-Design Scoping Studies will also provide more-defined project scopes, cost estimates for possible alternatives, potential environmental impacts, and other information that will be of assistance in the Phase I Design process. This study was done for Item Numbers 4-1068.00 and 4-1069.00 which are bridge replacement projects on US 150 in Nelson and Washington Counties. A handout of the meeting presentation was given to all meeting attendees. A sign-in sheet was also passed around.

**NINE ELEMENTS OF A PURPOSE AND NEED STATEMENT:** A checklist of the nine elements was displayed and the importance of each of the elements as they relate to the subject projects was discussed:

**Legislation** – The Design and Right-of-Way phases are scheduled in the 2010 Highway Plan. They are both funded with BRO funding. The description in the Highway Plan states that the bridges are to be replaced.

**Project Status** – Both the Bridges are structurally deficient. Bridge 090B00028N has a SR of 45.8, and Bridge 115B00022N has a SR of 41.1. Design funds have not yet been authorized. The Highway Plan design year is 2010. The Right of Way phase is scheduled for 2012. The district is unsure if the design of the approaches will be done inhouse.

System Linkage – US 150 in this area connects Springfield to Bardstown. It is a route used by truck traffic coming off of the Bluegrass Parkway. St. Catharine College is also on this route. The project team stated that the completion of US 150 in Rockcastle County may increase traffic from I-75. The road classifications of US 150 in the project area was discussed.

*Modal Interrelationships* – There is no public transit on this route. The nearest Rail Line is RJ Corman in Bardstown. The amount of traffic generated on this route by the Rail Line is unknown, but is not thought to be substantial. The project team does not believe that separate bike/pedestrian facilities are needed in this area.

**Social Demands & Economic Development** – There is a park located just southeast of the project site. There is another route into the park area. The greatest potential for development that would impact the project site is a 200 acre industrial park on the south side of the Bluegrass Parkway in Bardstown. Currently, there is a bakery there with more room for development.

**Transportation Demand** – Since no design money is currently authorized, traffic forecasts were not requested. Traffic projections are based on historic trends for this road. This section of US 150 has generally followed a 3% annual growth rate. The current ADT is approximately 8,500. If the historic 3% growth rate continues, the anticipated 2030 ADT will be near 15,000.

Capacity – According to the Division of Planning's data, the current V/SF is 0.46. If traffic volumes continue to follow a 3% growth rate, consideration may need to be given to increasing the number of through lanes on this corridor to accommodate the 2030 projection. There is a project in the UPL that is supported by local officials in Washington County to add lanes to this road.

Safety – Collision data was obtained from the KY State Police database of collisions for a three year period of time from June 1, 2007 to May 31, 2010. There were 12 collisions reported in the project area during this three year period of time. Four of the collisions were located at the intersection with Connor Road. Two were located at the intersection with Croakes Station Road. All but one of these occurred at night and, in the description of the collisions in the reports, two of them stated that sight distance was limited by the bridge railings. The project team agreed that this is more of a problem at night because

the bridge rail blocks the headlights of the oncoming vehicles at these intersections. The manner and location of other collisions were discussed. The project team did not believe that there is a significant traffic queue to turn into any of these entrances and turn lanes were not recommended.

Roadway Deficiencies – The roadway currently has 11 feet lanes, 4-8 feet shoulders with guardrail on both sides of the road, approximately a 0% grade, a posted speed limit of 55 MPH, and an Adequacy Rating Percentile of 85.7. KYTC's Common Geometric Practices for this type of road recommends 12 feet lanes for a 60 MPH Design Speed and 8 feet shoulders. Both bridges are structurally deficient with a rating of "Poor" for their Superstructure. Both bridges are between 27 to 28 feet wide, curb to curb. It should also be noted that there is a 46 ft. long, three-span culvert located approximately 500 feet west of the bridge over Beech Fork. The culvert is dry most of the time, and is used to accommodate the overflow from Beech Fork. It is not structurally deficient, but does have some issues with the wing walls separating from the culvert and some rebar exposure.

David Kemper, D4 Structures, stated that he is not aware of the bridges flooding, but water has risen to the superstructure and there is a problem with conveyance. There is a problem with debris catching on the piers in this location. The opening will need to be studied hydraulically during Phase I Design. It was suggested that the alignment be raised to increase the size of the hydraulic opening. Moving the pier to allow for a longer span (currently 90 feet) may also be helpful.

ENVIRONMENTAL CONSIDERATIONS: The bridges cross over Beech Fork and Cartwright Creek, which are blue line streams. It was noted during the site visit that the streams may contain a threatened species of Mussels. There is also some indication that there are wetlands located just southwest of the bridge over Beech Fork. The flood plain will need to be considered. The bridges are stamped as being built in 1955 and may be historically significant. According to the project team, the school located at the corner of Connor Road and Fredericktown Road in the GIS database is no longer open. Joseph Ferguson, D4 Environmental Coordinator, stated that there will be 6(f) issues with Fredericktown Park, which is adjacent to the project site. An EA will probably be required for this project. Joseph agreed to write a short Environmental Overview to include in the study report.

<u>UTILITIES:</u> A list of utility providers and contact information was given to Jill by John Edwards, D4 Utilities. The location of the overhead lines was noted during the site visit. The project team confirmed that there are no gas or sewer lines near the project site. Someone mentioned the possibility of a fiber optic cable in the area, but no markers could be seen during the site visit.

**POSSIBLE OPTIONS:** The following are some of the alternates that were discussed:

- No Build not a feasible option due to the structural deficiency of the bridges
- Build in Place

- o Temporary Crossing At the site visit it was noted that the terrain is not favorable for a low-water crossing.
- o Detour A detour using state routes would require motorists to go several miles out of their way.
- Move the Alignment north or south of the existing structures
  - o Moving the alignment to the south would have greater impacts to utilities, would impact Fredericktown Park creating 6(f) issues, and possibly have a much greater impact on Beech Fork and wetlands near the roadway than moving the alignment to the north.
  - There were a couple of options discussed to move the alignment to the north of the existing alignment:
    - Moving the new structure several feet north of the existing alignment to create a separate structure. This would require an extension of the culvert west of the bridges to accommodate the tie-in of the approaches to the new bridge. The culvert is not currently structurally deficient, but does have some issues with separation of the headwalls from the culvert and some exposure of rebar. These issues can be addressed if the culvert is extended. In addition, it is suggested that the alignment be raised to increase the hydraulic opening of the bridges. It was also recommended that current design standards be used (12 ft. lanes, 8 ft. shoulders) on both the approaches and the bridges, which would require the bridge to be 40 ft. curb to curb. The district did not recommend widening the bridge to accommodate any potential future widening of the roadway.

This option would allow for 2 lanes of traffic to remain open while constructing the bridges.

Another option is partial width construction of the new bridge which would shift the center line approximately 7 feet to the north in order to accommodate the proposed lane widths and shoulder widths of 12 feet and 8 feet, respectively. This would allow shorter tie-ins to the approaches, and would probably eliminate the need to extend the culvert. Raising the elevation of the alignment would still be possible.

This option would most likely have the least impact on right of way, but would require the road width to be reduced to one-lane during construction with a temporary traffic signal to control the direction of traffic flow. The width needed for traffic is 17 feet (12 feet lane width + 2 feet for the barrier + 3 feet for the overhang).

OTHER ISSUES: There are three field entrances and two entrances to county roads, Croakes Station Road and Connor Road, in the project area, next to the end of the bridges that will need to be considered. Recommended widening of the shoulders should allow for greater sight distance for cars pulling out of these entrances onto US 150.

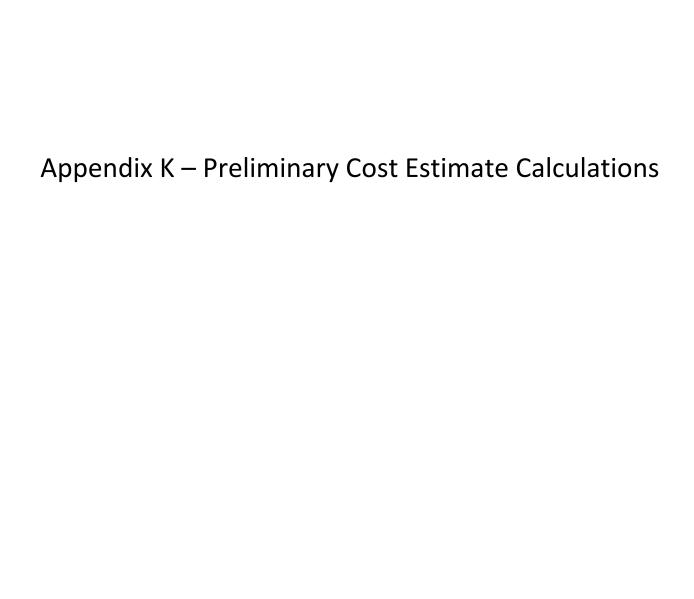
**PURPOSE & NEED:** After some discussion the project team agreed that the purpose and need statement should read similar to the following:

US 150 provides a vital connection between the city of Bardstown and Springfield. Bridges located over Beech Fork on the Nelson/Washington County Line and the bridge over Cartwright Creek just east of the County Line are structurally deficient. There are collisions occurring at the intersections of Croakes Station Road and Connor Road due to poor visibility caused by the bridge railings. There are also conveyance problems with the existing structures and the bridge piers accumulate large amounts of debris. The purpose of this project is to address the structural deficiencies and conveyance issues of the bridges and the occurrence of collisions at the intersections in order to provide safety, mobility and connectivity between the areas of Springfield and Bardstown.

**NEXT STEPS:** The district agreed to provide planning level estimates for the alternates they would like to see move forward. They will provide estimates for the approaches, but the estimate for the structures would be a square foot cost provided by the Division of Structural Design. The project team recommended that other roadway projects near the site and UPL projects in the area be included in the report. The interchange at the Bluegrass Parkway and the Springfield Bypass are the nearest projects. It was also requested that Jill check and see if any of the PVA information for the site is available online and that the vertical climb on the Nelson County side of the project be mentioned in the report.

Jill stated that she plans on having a draft report available by Mid-August. The meeting was followed by a visit to the site.

## **END OF MINUTES**



Project: 4-1068 & 4-1069

Alternate #1	Deck Area (sq. yds)		(sq. ft.)	\$/sq. ft.	Cost
4-1068 Bridge:	1980		17820	\$110	\$1,960,200
4-1069 Bridge:	1110		9990	\$110	\$1,098,900
	<u>Unit</u>	Quantity		Unit Price	
New Box Culvert:	LF	50		\$6,000	\$300,000
Remove Structures	LS	1		\$200,000	\$200,000
Asphalt Surf. (1 1/4")	Tons	1000		\$65	\$65,000
Aspahlt Base (9")	Tons	7000		\$65	\$455,000
DGA (6")	Tons	4600		\$20	\$92,000
Embankment	Cu. Yds	11000		\$10	\$110,000
MOT (3%)		3.00%			\$128,433
Mobilization (5%)		5.00%			\$214,055
Demob (1.5%)		1.50%			\$64,217
30% Contingency		30.00%			\$1,284,330
			Cons	truction Total:	\$5,972,135

Alternate #2 or 3	Deck Area (sq. yds)		(sq. ft.)	\$/sq. ft.	Cost		
4-1068 Bridge:	1980		17820	\$110	\$1,960,200		
4-1069 Bridge:	1110		9990	\$110	\$1,098,900		
	<u>Unit</u>	Quantity		<u>Unit Price</u>			
Box Culvert Ext.	LF	20		\$6,000	\$120,000		
Remove Structures	LS	1		\$200,000	\$200,000		
Asphalt Surf. (1 1/4")	Tons	170		\$65	\$11,050		
Aspahlt Base (9")	Tons	1200		\$65	\$78,000		
DGA (6")	Tons	900		\$20	\$18,000		
Embankment	Cu. Yds	2000		\$10	\$20,000		
MOT (10%)		10.00%			\$350,615		
Mobilization (5%)		5.00%			\$175,308		
Demob (1.5%)		1.50%			\$52,592		
30% Contingency		30.00%			\$1,051,845		
	<b>Construction Total:</b> \$5,136						